

PLANT LIST		
ORNAMENTAL TREES		
CODE	BOTANICAL / COMMON NAME	
CCJM	Carpinus caroliniana "J.N. Strain" / J.N. Strain American Hornbeam	
CECA	Cercis canadensis "Columbus" / Columbus Strain Eastern Redbud	
MAAM	Mascula amurensis / Amur Mascula	
OVERSTORY DECIDUOUS TREES		
CODE	BOTANICAL / COMMON NAME	
ACWM	Acer miyabei "Morton" / State Street™ Miyabe Maple	
COPR	Celtis occidentalis "Prairie Pride" / Prairie Pride Hackberry	
GBV	Ginkgo biloba "Autumn Gold"™ / Autumn Gold Maidenhair Tree	
GBPS	Ginkgo biloba "Princeton Sentry" / Princeton Sentry Maidenhair Tree	
GLTR	Gleditsia triacanthos inermis "Shademaster"™ / Shademaster Locust	
GIDE	Gymnocladus dioica "Espresso" / Kentucky Coffeetree	
PAMC	Platanus x acerifolia "Morton Circle" / Exclamation™ London Plane Tree	
UPRIGHT EVERGREEN SHRUB		
CODE	BOTANICAL / COMMON NAME	
THOC	Thuja occidentalis "Emerald" / Emerald Arborvitae	
THTE	Thuja occidentalis "Techny" / Techny Arborvitae	
DECIDUOUS SHRUBS		
CODE	BOTANICAL / COMMON NAME	
ARWE	Aronia melanocarpa "Morton"™ / Inquis Beauty Black Chokeberry	
CLAH	Clethra alnifolia "Hummingbird" / Hummingbird Summersweet	
COSE	Cornus sericea "Farrow" / Arctic Fire® Red Twig Dogwood	
FOVB	Foraythia villosum "Bronze" / Bronx Forsythia	
HPDP	Hydrangea paniculata "Dip Pinky"™ / Pinky Winky Panicle Hydrangea	
HLQI	Hydrangea paniculata "Little Quick Fire" / Little Quick Fire Hydrangea	
SPYA	Syringa x "SMURBY"™ / Blooming Dear® Pink Lilac	
VLJU	Viburnum dentatum "XLMeventide" / Little Joe™ Arrowwood Viburnum	
EVERGREEN SHRUBS		
CODE	BOTANICAL / COMMON NAME	
BLVJ	Buxus x "Green Velvet" / Green Velvet Boxwood	
JCPW	Juniperus chinensis "Pfitzeriana Kallag Compacta" / Kally Pfitzer Compact Juniper	
JUCH	Juniperus chinensis "Sea Green" / Sea Green Juniper	
JUMA	Juniperus sabina "Mini-Arcadia" / Mini Arcadia Juniper	
PERENNIALS & GRASSES		
CODE	BOTANICAL / COMMON NAME	
ALSU	Allium x "Summer Beauty" / Summer Beauty Allium	
AOCA	Aquilegia canadensis / Eastern Columbine	
ASAZ	Aster azureus / Sky Blue Aster	
CACI	Calamagrostis x occiflora "Walt Foerster" / Karl Foerster Feather Reed Grass	
CANE	Calamagrostis repens "Montrose White" / Montrose White Catmint	
ECPU	Echinacea purpurea "PASTEL217" / Pastel® Wild Berry Coneflower	
LISK	Liatris spicata "Kobold" / Kobold Spike Gayfeather	
MNPT	Monarda fistulosa / Bergamot	
PAVI	Panicum virgatum "Shenandoah" / Shenandoah Switch Grass	
PINW	Panicum virgatum "Northwind" / Northwind Switch Grass	
PNBT	Penstemon x "Dark Towers" / Dark Towers Beardtongue	
RUFU	Rudbeckia fulgida sulfurea "Goldsturm" / Goldsturm Coneflower	
SCSC	Schizanthus scaberrimus "Prairie Blues" / Prairie Blues Little Bluestem	
SPVE	Sporobolus heterolepis "Tara" / Prairie Droopseed	

- CONTRACTOR NOTES**
- ALL LANDSCAPE AREAS TO RECEIVE SHREDDED HARDWOOD BARK MULCH UNLESS OTHERWISE SPECIFIED.
 - REFERENCE SHEET C1.0 FOR LEGEND.



1010 East Wisconsin Avenue,
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608 / 242 1550

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CONSULTANTS:

CLIENT:

Community Development Authority of the
City of Madison

Madison Municipal Building
215 Martin Luther King Jr Blvd
Suite 161
Madison, WI 53703

PROJECT TITLE:

VILLAGE ON PARK PARKING
STRUCTURE AND SITE
IMPROVEMENTS

808 HUGHES PLACE
MADISON, WI 53713

ISSUE:

07/28/2023 BID DOCUMENTS

PROJECT INFORMATION:

PROJECT NUMBER: 20225013.00

DATE: 07/28/2023

DRAWN BY: MRA

CHECKED BY: KJY

APPROVED BY: KJY

AS NOTED

SHEET TITLE:

LANDSCAPE PLAN -
OVERALL

SHEET NUMBER:

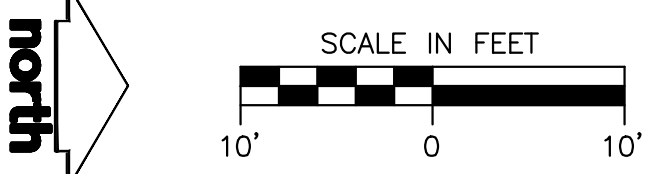
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VILLAGER SHOPPING CENTER

PLANT LIST		
ORNAIMENTAL TREES		
CODE	BOTANICAL / COMMON NAME	
CCJN	Carpinus caroliniana 'J.N. Strain' / J.N. Strain American Hornbeam	
CEEA	Cercis canadensis 'Columbus' / Columbus Strain Eastern Redbud	
MAAM	Mascula amurensis / Amur Mascula	
OVERSTORY DECIDUOUS TREES		
CODE	BOTANICAL / COMMON NAME	
ACJW	Acer myriophyllum 'Morton' / State Street™ Myriophyllum	
COPP	Celtis occidentalis 'Prairie Pride' / Prairie Pride Hackberry	
GIBI	Ginkgo biloba 'Autumn Gold'™ / Autumn Gold Maidenhair Tree	
GBPS	Ginkgo biloba 'Princeton Sentry' / Princeton Sentry Maidenhair Tree	
GLTR	Gleditsia triacanthos inermis 'Shademaster'™ / Shademaster Locust	
GYDE	Gymnocladus dioica 'Espresso' / Kentucky Coffee Tree	
PLAM	Platanus x acerifolia 'Morton Circle' / Exclamation™ London Plane Tree	
UPRIGHT EVERGREEN SHRUB		
CODE	BOTANICAL / COMMON NAME	
HOOC	Thuja occidentalis 'Emerald' / Emerald Arborvitae	
THTE	Thuja occidentalis 'Techny' / Techny Arborvitae	
DECIDUOUS SHRUBS		
CODE	BOTANICAL / COMMON NAME	
ARNE	Aronia melanocarpa 'Morton'™ / Inquis Beauty Black Chokeberry	
CLAH	Clethra alnifolia 'Hummingbird' / Hummingbird Summersweet	
COSE	Cornus sericea 'Forsyth' / Arctic Fire® Red Twig Dogwood	
FOVB	Foraythia villosoidea 'Bronzeville' / Bronze Forsythia	
HPLD	Hydrangea paniculata 'Shag Pink'™ / Pinky Winky Panicle Hydrangea	
HYLG	Hydrangea paniculata 'Little Quick Fire' / Little Quick Fire Hydrangea	
SYPA	Syringa x 'SAMPLERS'™ / Blooming Dear® Pink Lilac	
VLJU	Viburnum dentatum 'XLMexcellent' / Little Joe™ Arrowwood Viburnum	
EVERGREEN SHRUBS		
CODE	BOTANICAL / COMMON NAME	
BUOV	Buxus x 'Green Velvet' / Green Velvet Boxwood	
JCPW	Juniperus chinensis 'Pfitzerana Kallia Compacta' / Kallia Pfitzer Compact Juniper	
JUCH	Juniperus chinensis 'Sea Green' / Sea Green Juniper	
JUMA	Juniperus sabina 'Mini-Arcadia' / Mini Arcadia Juniper	
PERENNIALS & GRASSES		
CODE	BOTANICAL / COMMON NAME	
ALSU	Allium x 'Summer Beauty' / Summer Beauty Allium	
AGCA	Aquilegia canadensis / Eastern Columbine	
ASAZ	Aster azureus / Sky Blue Aster	
CACI	Colamagrostis x occidens 'Walt Foerster' / Walt Foerster Feather Reed Grass	
CANE	Colomintha nepeta 'Monrose White' / Monrose White Catmint	
ECPU	Echinacea purpurea 'PASTEL2017' / Pastel® Wild Berry Coneflower	
LISS	Liatris spicata 'Wobolt' / Kobold Spike Gayfeather	
MNFI	Monarda fistulosa / Bergamot	
PAXI	Panicum virgatum 'Shenandoah' / Shenandoah Switch Grass	
PVNW	Panicum virgatum 'Northwind' / Northwind Switch Grass	
PNDT	Parthenocera x 'Dark Towers' / Dark Towers Beardtongue	
RUFU	Rudbeckia fulgida var. 'Goldsturm' / Goldsturm Coneflower	
SCSC	Schizanthus luteus 'Prairie Blues' / Prairie Blues Little Bluestem	
SPHE	Sporobolus heterolepis 'Tara' / Prairie Droopseed	

PLANT SCHEDULE - STREET TREES S. PARK ST.				
STREET TREES	CODE	BOTANICAL / COMMON NAME	CONT.	QTY
	CEOC	Celtis occidentalis Common Hackberry	B & B	2"Cal 1
	PLAM	Platanus x acerifolia 'Morton Circle' Exclamation™ London Plane Tree	B & B	2"Cal 2
	ULNH	Ulmus x 'New Horizon' New Horizon Elm	B & B	2"Cal 2

- CONTRACTOR NOTES
- ALL LANDSCAPE AREAS TO RECEIVE SHREDDED HARDWOOD BARK MULCH UNLESS OTHERWISE SPECIFIED.
 - SUBSURFACE UTILITIES HAVE BEEN INTENTIONALLY REMOVED FROM 10' SCALE SHEETS FOR PLAN LEGIBILITY. CONTRACTOR SHALL REFER TO L100 AND DETAILED CIVIL SHEETS FOR UTILITIES PRIOR TO PLANTING INSTALLATION.



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AS NOTED

SHEET TITLE:

LANDSCAPE PLAN -
VILLAGE GREEN SOUTH

SHEET NUMBER:

L102

CONTRACTOR NOTES

1. ALL LANDSCAPE AREAS TO RECEIVE SHREDDED HARDWOOD BARK MULCH UNLESS OTHERWISE SPECIFIED.
2. SUBSURFACE UTILITIES HAVE BEEN INTENTIONALLY REMOVED FROM 10' SCALE SHEETS FOR PLAN LEGIBILITY. CONTRACTOR SHALL REFER TO L100 AND DETAILED CIVIL SHEETS FOR UTILITIES PRIOR TO PLANTING INSTALLATION.

1. ALL LANDSCAPE AREAS TO RECEIVE SHREDDED HARDWOOD BARK MULCH UNLESS OTHERWISE SPECIFIED.
2. SUBSURFACE UTILITIES HAVE BEEN INTENTIONALLY REMOVED FROM 10' SCALE SHEETS FOR PLAN LEGIBILITY. CONTRACTOR SHALL REFER TO L100 AND DETAILED CIVIL SHEETS FOR UTILITIES PRIOR TO PLANTING INSTALLATION.



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DESIGN SPECIFICATIONS

- DESIGN IS IN ACCORDANCE WITH THE STATE OF WISCONSIN AND THE 2015 INTERNATIONAL BUILDING CODE.
- MINIMUM 28 DAY CONCRETE CYLINDER STRENGTH SHALL BE:

FOOTINGS	4000 PSI
PIERS	4000 PSI
GRADE BEAMS	4000 PSI
FOUNDATION WALLS	4000 PSI
SLABS ON GRADE	4000 PSI
COLUMNS	5000 PSI
STRUCTURAL SLAB SYSTEMS	6000 PSI

- REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60.
- POST-TENSIONING STEEL SHALL CONFORM TO ASTM A416.
- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 TYPE II NORMAL WEIGHT UNITS
- CONCRETE MASONRY BRICK SHALL CONFORM TO ASTM C55 GRADE SW
- MORTAR SHALL CONFORM TO ASTM C270.
- MASONRY GROUT SHALL CONFORM TO ASTM C476. MINIMUM COMPRESSIVE STRENGTH SHALL BE $f_m = 3000$ PSI.
- MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION SHALL BE $f_m = 2500$ PSI.
- STRUCTURAL STEEL W-SHAPES SHALL CONFORM TO ASTM A992 GRADE 50.
- STRUCTURAL STEEL PLATES, ANGLES, CHANNELS, AND OTHER ROLLED MEMBERS SHALL CONFORM TO ASTM A36.
- RECTANGULAR OR SQUARE HSS MEMBERS SHALL CONFORM TO ASTM A500 GRADE B.
- ROUND HSS MEMBERS SHALL CONFORM TO ASTM A500 GRADE B.
- STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B.

- ASSUMED BEARING CAPACITY FOR SPREAD FOOTINGS IS 5000 PSF UNLESS NOTED OTHERWISE, BASED UPON INSTALLATION OF RAMMED AGGREGATE PIERS AS DESIGNED BY RAMMED AGGREGATE PIER INSTALLER.

DESIGN LOADS: (PARKING GARAGE)

FLOOR LIVE LOADS	
GARAGES	40 PSF
STAIRS AND EXIT WAYS	100 PSF
LOBBIES	100 PSF
MECHANICAL ROOMS	50 PSF
ROOF LIVE LOAD	
MINIMUM ROOF LIVE LOAD	20 PSF
LIVE LOAD REDUCTION	
LIVE LOAD REDUCTION PER IBC 2015 SECTION 1607.10 IS INCLUDED.	

ROOF SNOW LOAD (ASCE 7-10)	
RISK CATEGORY	II
IMPORTANCE FACTOR	$I_s = 1.0$
GROUND SNOW LOAD	$P_g = 30$ PSF
FLAT ROOF SNOW LOAD	$P_f = 25.2$ PSF
EXPOSURE FACTOR	$C_e = 1.0$
THERMAL FACTOR	$C_t = 1.2$

REFER TO PLAN SHEETS FOR SNOW DRIFT SURCHARGE LOADS (P_D) AND WIDTHS OF SNOW DRIFTS (w)

ROOF RAIN LOAD
BUILDING HAS BEEN DESIGNED FOR RAIN LOADS PER IBC 2015 SECTION 1611.

WIND LOAD (ASCE 7-10)	
RISK CATEGORY	II
ULTIMATE WIND SPEED	$V_{ult} = 115$ MPH
NOMINAL WIND SPEED	$V_{ref} = 90$ MPH
EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT	$GCF_p = +/- 0.55$
COMPONENTS AND CLADDING	REFER TO TABLE THIS SHEET

SEISMIC LOAD (IBC 2015)	
RISK CATEGORY	II
IMPORTANCE FACTOR	$I_s = 1.0$
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS	$S_a = 0.085$ g
DESIGN SPECTRAL RESPONSE ACCELERATION	$S_d = 0.046$ g
SEISMIC RESPONSE COEFFICIENT	$S_{ps} = 0.091$ g
RESPONSE MODIFICATION FACTOR	$C_d = 0.01$
SITE CLASS	D
SEISMIC DESIGN CATEGORY	A
SEISMIC FORCE-RESISTING SYSTEM	ORDINARY REINFORCED CONCRETE MOMENT FRAMES
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PROCEDURE
DESIGN BASE SHEAR	150 KIPS (N-S/E-W)

DESIGN LOADS: (EXISTING GROCERY)

ROOF LIVE LOAD	
MINIMUM ROOF LIVE LOAD	20 PSF
ROOF SNOW LOAD (ASCE 7-10)	
RISK CATEGORY	II
IMPORTANCE FACTOR	$I_s = 1.0$
GROUND SNOW LOAD	$P_g = 30$ PSF
FLAT ROOF SNOW LOAD	$P_f = 25.2$ PSF
EXPOSURE FACTOR	$C_e = 1.0$
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ROOF RAIN LOAD	
BUILDING HAS BEEN DESIGNED FOR RAIN LOADS PER IBC 2015 SECTION 1611.	
WIND LOAD (ASCE 7-10)	
RISK CATEGORY	II
ULTIMATE WIND SPEED	$V_{ult} = 115$ MPH
NOMINAL WIND SPEED	$V_{ref} = 90$ MPH
EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT	$GCF_p = +/- 0.18$
COMPONENTS AND CLADDING	REFER TO TABLE THIS SHEET

- RESISTANCE TO LATERAL LOADS ON STRUCTURE IS PROVIDED BY FLOOR DIAPHRAGMS. CONTRACTOR SHALL PROVIDE SUFFICIENT TEMPORARY BRACING UNTIL ALL LATERAL SUPPORT SYSTEMS ARE IN PLACE AND FUNCTIONAL.

- ALL STRUCTURAL FRAMING AND CONNECTIONS HAVE BEEN DESIGNED FOR THE FINAL COMPLETED CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADINGS ENCOUNTERED DURING ERECTION AND CONSTRUCTION. ANY INVESTIGATION OF THE STRUCTURAL FRAMING AND CONNECTIONS FOR ADEQUACY DURING THE ERECTION AND CONSTRUCTION PROCESS IS THE RESPONSIBILITY OF THE CONTRACTOR.

- CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION AND JOB SITE SAFETY.

GENERAL NOTES

EARTHWORK

- FOOTINGS SHALL BE CAST ON UNDISTURBED SUBSOIL. IF DESIGN CAPACITY IS NOT ENCOUNTERED AT THE ELEVATIONS SHOWN, FOOTINGS MUST BE LOWERED. CONSULT ENGINEER BEFORE PROCEEDING.
- NO HOLES, TRENCHES OR DISTURBANCES OF THE SOIL SHALL BE ALLOWED WITHIN THE VOLUME DESCRIBED BY 45 DEGREE LINES SLOPING FROM THE BOTTOM EDGE OF THE FOOTING. IF SUCH ARE REQUIRED, FOOTINGS MUST BE LOWERED.
- BACKFILL EVENLY ON EACH SIDE OF FOUNDATION WALLS AND RETAINING WALLS.
- DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL FLOOR SYSTEM IS IN PLACE AND FASTENED OR UNTIL WALLS ARE ADEQUATELY BRACED. BRACING SHALL BE DESIGNED BY THE CONTRACTOR
- TOPSOIL AND FILL BELOW SLABS ON GROUND SHALL BE REMOVED. AGGREGATE BASE COURSE UNDER SLABS ON GROUND SHALL BE AS SPECIFIED (EXCEPT WHERE LOOSE FILL IS INDICATED ON DRAWINGS).
- BACKFILL AGAINST EXTERIOR FOUNDATION WALLS SHALL BE AS SPECIFIED COMPACTED TO MAXIMUM 6-INCH LAYERS.
- BACKFILL AGAINST EXTERIOR FOUNDATION WALLS SHALL BE AS SPECIFIED COMPACTED TO MAXIMUM 6-INCH LAYERS.
- PROVIDE MINIMUM 24 INCHES OF FREE DRAINING AGGREGATE OVER ALL DRAIN TILES AND 4 INCHES BELOW.

CONCRETE

- FORMWORK SHALL BE DESIGNED IN ACCORDANCE WITH THE ACI "MANUAL OF CONCRETE PRACTICE", LATEST EDITION.
- REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH THE ACI "MANUAL OF CONCRETE PRACTICE", LATEST EDITION, UNLESS OTHERWISE NOTED.
- LAP ALL WALL BARS 30 DIAMETERS WITH CLASS B SPLICES UNLESS OTHERWISE DETAILED. LAP WELDED WIRE MESH 6 INCHES.
- PROVIDE COLUMN AND WALL DOWELS OF THE SAME SIZE AND NUMBER AS THE RESPECTIVE COLUMN AND WALL REINFORCING UNLESS OTHERWISE DETAILED.
- PROVIDE TWO #4 BARS AS STIRRUP CARRY BARS WHERE NO TOP STEEL IS AVAILABLE TO HOLD STIRRUPS.
- WHEREVER AN APPROVED PIPE OR CONDUIT EXTENDS THROUGH A BEAM, PROVIDE ONE ADDITIONAL STIRRUP ON EACH SIDE OF THE OPENING.
- CONCRETE PROTECTION FOR REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI 318-14.
- SLABS ON GRADE SHALL BE CAST ALLOWING A SUFFICIENT NUMBER OF JOINTS TO ADEQUATELY CONTROL SHRINKAGE CRACKING. SAWCUTTING SHALL BE DONE AS SOON AS SAWCUT WILL NOT RAVEL. CONCRETE OR WITHIN 24 HOURS MAXIMUM OF INITIAL POURING OPERATION. MAXIMUM SIZE OF PANELS SHALL BE 15 FEET BY 15 FEET FOR 5-INCH SLAB ON GRADE. GENERALLY, JOINTS SHALL OCCUR ON COLUMN CENTERLINES.
- SLABS ON GRADE SHALL BE 5 INCHES THICK AND REINFORCED AS SHOWN ON THE DRAWINGS.
- ALLOW AT LEAST 24 HOURS BEFORE POURING ADJACENT WALL SECTIONS BETWEEN CONSTRUCTION JOINTS. MAXIMUM LENGTH OF POUR TO BE 40 FEET, UNLESS CRACK INDUCERS ARE USED AS DETAILED ON THE DRAWINGS.
- CONTRACTOR SHALL NOTIFY THE ENGINEER AT LEAST 24 HOURS PRIOR TO PLACING CONCRETE.
- CONCRETE SLABS ON METAL DECK SHALL BE POURED TO A CONSTANT THICKNESS.
- CONSTRUCTION JOINTS IN BEAMS, JOISTS OR SLABS TO BE LOCATED BETWEEN THE 1/4 POINT AND CENTERLINE OF SPAN, OR AS DIRECTED BY THE ENGINEER.
- DO NOT PLACE OR CUT HOLES IN CONCRETE SLABS, BEAMS, WALLS OR COLUMNS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- EXTERIOR EXPOSED CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 6 PERCENT (+/- 1 1/2 PERCENT).
- CAMBER CONCRETE MEMBERS FOR DEAD LOAD DEFLECTION BY ADJUSTING FORMS.
- PIPES AND CONDUITS EMBEDDED IN OR PASSING THROUGH STRUCTURAL MEMBERS MUST BE APPROVED BY THE STRUCTURAL ENGINEER. PIPE AND CONDUITS EMBEDDED IN CONCRETE SHALL NOT BE LARGER THAN 2 INCHES IN OUTSIDE DIAMETER AT THEIR WIDEST POINT OR FITTING OR 1/3 OF THE THICKNESS OF THE SLAB, BEAM OR WALL.
- ELECTRICAL CONDUIT OR PIPES EMBEDDED IN OR PASSING THROUGH SLABS, BEAMS OR WALLS SHALL BE LOCATED AND PLACED SO THAT:
 - THEY ARE NOT CLOSER THAN THREE DIAMETERS ON CENTER.
 - THE CONCRETE COVER IS NOT LESS THAN 2 INCHES.
 - THEY RUN BETWEEN REINFORCING AND DO NOT DISPLACE IT IN ANY MANNER.
- ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE.
- CHAMFER ALL EXPOSED CONCRETE CORNERS. SEE ARCHITECTURAL/STRUCTURAL DRAWINGS FOR REQUIREMENTS.
- CONCRETE SHALL BE TESTED BY THE OWNER'S TESTING LAB. REFER TO SPECIFICATIONS FOR REQUIREMENTS.
- PROPER CURING PROCEDURES SHALL BE USED FOR SLAB ON GRADE TO PREVENT CURLING.
- CALCIUM CHLORIDE SHALL NOT BE USED IN CONCRETE MIXES.
- PROVIDE WATERSTOPS AT ALL CONSTRUCTION JOINTS BELOW THE WATER TABLE AND AS SHOWN ON DRAWINGS. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.

CONCRETE COVER REQUIREMENTS FOR MILD REINFORCEMENT (MINIMUM)

COMPONENT	REQUIRED COVER
FOOTINGS	3"
BEAMS (BOTTOM)	1 1/2"
BEAMS (TOP)	2"
SLABS (BOTTOM)	1"
SLABS (TOP)	2"
WALLS (EXTERIOR FACE)	2"
COLUMN TIES	2"

CONCRETE MASONRY

- PRODUCTION AND CONSTRUCTION OF CONCRETE MASONRY SHALL BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", ACI 530-13, AND THE NOMA "TEK MANUAL FOR CONCRETE MASONRY DESIGN AND CONSTRUCTION", LATEST EDITION.
- HOT AND COLD WEATHER CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE IMAC (INTERNATIONAL MASONRY INDUSTRY ALL-WEATHER COUNCIL) "RECOMMENDED PRACTICES AND GUIDE SPECIFICATIONS FOR HOT AND COLD WEATHER MASONRY AND CONSTRUCTION".
- CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- MASONRY WALLS SHALL BE ADEQUATELY BRACED TO RESIST WIND FORCES UNTIL PERMANENT DESIGN SUPPORTS ARE IN PLACE AND FUNCTIONAL. BRACING SHALL BE DESIGNED BY THE CONTRACTOR.
- PROVIDE DOWELS INTO FOUNDATION THE SAME SIZE AND NUMBER AS WALL REINFORCING.
- LAP REINFORCING BARS 48 DIAMETERS.
- CONCRETE MASONRY WALLS SHALL BE REINFORCED AT EVERY OTHER BED JOINT WITH 9 GAGE LADDER TYPE JOINT REINFORCEMENT.
- VERTICAL BARS SHOWN ON THE DESIGN DRAWINGS SHALL BE PLACED IN A CONTINUOUS UNOBSTRUCTED CELL OF NOT LESS THAN 3 INCHES BY 4 INCHES.
- ALL BOND BEAMS AND PILASTERS SHALL BE REINFORCED AS SHOWN ON THE DESIGN DRAWINGS AND FILLED WITH GROUT.
- ALL DOOR AND WINDOW JAMBS SHALL BE GROUTED SOLID 8 INCHES WIDE UNLESS SHOWN OTHERWISE.
- WHERE NOT SHOWN OTHERWISE, MINIMUM SOLID GROUTED MASONRY BELOW BEAM REACTIONS SHALL BE 16 INCHES DEEP BY 32 INCHES LONG.
- WHERE NOT SHOWN OTHERWISE, MINIMUM SOLID GROUTED MASONRY BELOW LINTEL REACTIONS SHALL BE 16 INCHES DEEP BY 16 INCHES LONG.

PRECAST CONCRETE

- PRECAST CONCRETE MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI 318-14.
- PRECAST CONCRETE SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE ACI "MANUAL OF CONCRETE PRACTICE", LATEST EDITION, AND THE AFOREMENTIONED CONCRETE PROVISIONS.
- PRECAST CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER DESIGN AND REINFORCING OF PRECAST CONCRETE FOR HANDLING AND ERECTION STRESSES.
- PRECAST MEMBERS SHALL BE ATTACHED AND SUPPORTED BY THE STRUCTURE AS INDICATED ON THE DRAWINGS.
- PRECAST MEMBERS SHALL BE DESIGNED AND REINFORCED FOR SELF-WEIGHT AND ALL SUPERIMPOSED LOADS SHOWN ON THE DRAWINGS.
- PRECAST MEMBERS SHALL BE CAPABLE OF SAFELY SUPPORTING ANY CONCENTRATED LOADS INDICATED BY THE STRUCTURAL, MECHANICAL, AND ARCHITECTURAL DRAWINGS.
- PRECAST CONTRACTOR SHALL FURNISH AND INSTALL ALL MATERIALS (HANGERS, CLIPS, PLATES, HEADERS, ANCHORAGES, ETC.) WHICH MUST BE PRECAST INTO THE CONCRETE UNLESS OTHERWISE NOTED OR REQUIRED FOR CONNECTION OF PRECAST TO STRUCTURE.
- CONTRACTOR SHALL COORDINATE LOCATIONS OF ALL HOLES OR OPENINGS WITH RESPECTIVE TRADES BEFORE FABRICATION. ANY DEVIATION FROM THESE LOCATIONS OR ADDITIONAL OPENINGS MUST BE APPROVED BY THE FABRICATOR.
- GROUT IN PRECAST MEMBER KEYWAYS SHALL BE SAND-CEMENT GROUT. MINIMUM COMPRESSIVE STRENGTH SHALL BE 2000 PSI.
- FIRE RATING OF PRECAST WALL PANELS SHALL BE 2 HOUR.
- WALL PANEL JOINTS SHALL BE FILLED WITH APPROVED FIRE STOP MATERIAL AND POLYURETHANE JOINT SEALANT.

STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE AISC "STEEL CONSTRUCTION MANUAL", FOURTEENTH EDITION, AND THE AISC "CODE OF STANDARD SPECIFICATION FOR STEEL BUILDINGS AND BRIDGES", APRIL 14, 2010 EDITION.
- WHERE INDICATED ON DRAWINGS, STRUCTURAL AND MISCELLANEOUS STEEL WHICH SHALL REMAIN EXPOSED TO VIEW SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR ARCHITECTUALLY EXPOSED STRUCTURAL STEEL", LATEST EDITION, WITHOUT GAPS OR OPEN JOINTS.
- STEEL JOIST FABRICATION AND ERECTION SHALL CONFORM TO THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE. JOISTS SHALL BE MANUFACTURED WITH HOT-ROLLED TOP AND BOTTOM CHORD MEMBERS.
- STEEL DECK FABRICATION AND ERECTION SHALL CONFORM TO THE STANDARD SPECIFICATIONS OF THE STEEL DECK INSTITUTE.
- ALL WELDING SHALL COMPLY WITH AWS D1.1 USING E70XX ELECTRODES. ALL WELDING TO BE DONE BY AWS PREQUALIFIED WELDERS, CERTIFIED FOR WELDS MADE. PROVIDE CONTINUOUS MINIMUM SIZED WELDS PER AISC REQUIREMENTS, UNLESS NOTED OTHERWISE.
- THE MINIMUM SIZE OF FILLET WELDS SHALL BE AS SPECIFIED IN TABLE J2.4 IN THE AISC "STEEL CONSTRUCTION MANUAL".
- MINIMUM STRENGTH OF WELDED CONNECTIONS: UNLESS NOTED OTHERWISE ON THE DRAWINGS, ALL SHOP AND FIELD WELDS SHALL DEVELOP THE FULL TENSILE STRENGTH OF THE MEMBER OF ELEMENT JOINED. ALL MEMBERS WITH MOMENT CONNECTIONS, NOTED ON THE DRAWINGS, SHALL BE WELDED TO DEVELOP THE FULL FLEXURAL CAPACITY OF THE MEMBER, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- BOLTED CONNECTIONS SHALL BE MADE WITH ASTM A325 HIGH STRENGTH BOLTS (MINIMUM 3/4-INCH DIAMETER).
BEAM-TO-COLUMN CONNECTIONS SHALL BE MADE WITH DOUBLE ANGLES UNLESS OTHERWISE DETAILED.
- MINIMUM NUMBER OF BOLTS FOR HED SHEAR REACTIONS ARE AS FOLLOWS:
 - W8, W10 OR W12: 2
 - W12, W14 OR W16: 3
 - W16, W18 OR W20: 4
 - W20, W24 OR W27: 5
 - W27, W30 OR W36: 6
 - W36, W40 OR W44: 7
- BEAMS AND JOISTS SHALL BE EQUALLY SPACED IN A BAY UNLESS NOTED OTHERWISE ON PLAN.
- ALL STRUTS, HANGERS, AND BRACES SHALL HAVE CONNECTIONS DESIGNED TO DEVELOP THE FULL ALLOWABLE TENSILE STRENGTH OF THE MEMBER UNLESS THE DESIGN FORCE IS INDICATED ON THE DRAWINGS, IN WHICH CASE THE CONNECTIONS SHALL BE DESIGNED FOR THE FORCE INDICATED.
- COLUMN BASE PLATES SHALL HAVE OVERSIZED HOLES WITH PLATE WASHERS (MINIMUM 3/8-INCH THICK) PROVIDED WITH ANCHOR RODS.
- GROUT UNDER BASE PLATES IN ACCORDANCE WITH THE "AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", APRIL 14, 2010 EDITION.
- WHERE BEAMS SUPPORT JOISTS FROM ONLY ONE SIDE, JOIST SEAT SHALL EXTEND 1-INCH BEYOND BEAM CENTERLINE.
- JOISTS SHALL BE DESIGNED FOR NET UPLIFT SHOWN ON THE "WIND PROVISIONS FOR COMPONENTS AND CLADDING TABLE" THIS SHEET. JOIST SUPPLIER SHALL PROVIDE ALL ADDITIONAL BRIDGING AND BRACING AS REQUIRED BY THE STEEL JOIST INSTITUTE.
- ALL JOISTS ADJACENT AND RUNNING PARALLEL TO BEAMS SHALL BE SUPPLIED WITH ONE HALF OF STANDARD CAMBER.
- STEEL ROOF DECK SHALL BE WIDE RIB 1 1/2-INCH DEEP AND 18 GAGE THICKNESS UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
- DECK END LAPS SHALL BE 2-INCH MINIMUM AND SHALL OCCUR AT SUPPORTS. LOCATE AT VALLEYS AND RIDGES.
- WHERE CONTINUOUS DIAPHRAGM CHORD ANGLES ARE INDICATED, PROVIDE A FULL PENETRATION WELD AT THE SPlice LOCATIONS.
- CLEAN, PREPARE, AND SHOP PRIME EXTERIOR EXPOSED STRUCTURAL STEEL MEMBERS IN ACCORDANCE WITH SSPC STANDARDS SP-1 AND SP-6.
- CLEAN, PREPARE, AND SHOP PRIME INTERIOR EXPOSED STRUCTURAL STEEL MEMBERS IN ACCORDANCE WITH SSPC STANDARDS SP-1 AND SP-3.
- WHILE THE DESIGN DOCUMENTS MAY REFERENCE OSHA, THEY ARE NOT INTENDED TO SPECIFICALLY IDENTIFY ALL APPLICABLE OSHA REQUIREMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IDENTIFY AND COMPLY WITH ALL APPLICABLE OSHA REQUIREMENTS.
- ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO THE WEATHER, INCLUDING MASONRY SHELF ANGLES, SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123, UNLESS OTHERWISE NOTED.
- REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL MISCELLANEOUS STEEL.

COLD-FORMED STEEL FRAMING

- DESIGN, FABRICATION, AND ERECTION OF COLD-FORMED STEEL FRAMING SHALL BE IN ACCORDANCE WITH THE AISI "COLD-FORMED STEEL DESIGN MANUAL", LATEST EDITION. ALL FRAMING MEMBERS SHOWN ON PLANS ARE SCHEMATIC AND ARE SHOWN FOR INTENT ONLY. DESIGN AND CALCULATIONS WILL BE REVIEWED BY GRAEF.
- STEEL STUD CURTAIN WALL AND CONNECTIONS TO BE DESIGNED BY SUPPLIER. STEEL STUD CURTAIN WALL AND CONNECTION DESIGN SHALL BE SEALED BY PROFESSIONAL ENGINEER EXPERIENCED IN THIS WORK AND REGISTERED IN THE STATE OF WISCONSIN.
- LIVE LOAD DEFLECTION CRITERIA FOR COMPONENTS SHALL BE AS FOLLOWS:

EXTERIOR WALL STUDS	L/240 NOT TO EXCEED 1-INCH AT METAL PANELS L/600 FOR BRICK VENEER
---------------------	--
- MINIMUM DESIGN THICKNESS OF STUDS AND TRACK AT EXTERIOR OF BUILDING SHALL BE 18 GAGE.
- LOAD BEARING STUDS SHALL BE DESIGNED TO CARRY ALL GRAVITY LOADS AND LATERAL FORCES INCLUDING BUT NOT LIMITED TO DEAD LOADS, LIVE LOADS, WIND LOADS, AND AXIAL LOAD ECCENTRICITY.
- NON-LOAD BEARING STUDS SHALL TRANSFER LATERAL LOADS TO STRUCTURE BY MEANS OF SLIDE CLIPS TO ALLOW FOR VERTICAL MOVEMENT OF PRIMARY STRUCTURAL MEMBERS.
- SPLICES IN AXIALLY LOADED STUDS ARE NOT PERMITTED.
- STUDS, TRACK, AND ACCESSORIES SHALL BE GALVANIZED WITH A MINIMUM G90 COATING PER ASTM A653.
- STUDS SHALL BE PLUMBED, ALIGNED, AND SECURELY ATTACHED TO FLANGES OR WEBS OF LOWER TRACK. STUDS SHALL BE SEATED TIGHT TO TRACK WEBS PRIOR TO ATTACHMENT.
- JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS OR A LOAD DISTRIBUTION MEMBER SHALL BE PROVIDED AT THE TOP OF THE WALL.
- REFER TO ARCHITECTURAL WALL SECTIONS AND DETAILS FOR ADDITIONAL INFORMATION.
- ALL MEMBERS 0.066-INCH MINIMUM THICKNESS OR THICKER (18 GAGE OR LOWER) SHALL BE OF MINIMUM 50 KSI STEEL. ALL MEMBERS OF 0.045-INCH MINIMUM THICKNESS OR THINNER (18 GAGE OR HIGHER) AND ALL ACCESSORIES SHALL BE OF MINIMUM 33 KSI STEEL.

- STEEL STUD ERECTOR SHALL CONSTRUCT ALL LIGHT-GAGE FRAMING IN A MANNER WHICH PROTECTS LATERAL STABILITY OF THE STRUCTURE.
- ALL WELDS PERFORMED ON GALVANIZED LIGHT-GAGE COMPONENTS SHALL BE COATED WITH ZINC RICH PAINT FOR CORROSION PROTECTION IN ACCORDANCE WITH ASTM A780. CONTRACTOR SHALL NOTIFY THE ENGINEER TO ALLOW ADEQUATE TIME FOR WELDS TO BE REVIEWED BEFORE SYSTEMS ARE ENCLOSED.
- STEEL STUD WALLS SHALL BE DESIGNED AND CONSTRUCTED TO PROVIDE REQUIRED CAPACITIES TO CARRY CONSTRUCTION LOADS. CONTRACTOR SHALL PROVIDE NECESSARY BRIDGING OR ATTACHMENT TO WALL SHEATHING BEFORE STRUCTURAL COMPONENTS ARE LOADED.

MISCELLANEOUS

- DIMENSIONS OF EXISTING CONSTRUCTION OR CONSTRUCTION IN PROGRESS SHALL BE VERIFIED AND COORDINATED PRIOR TO FABRICATION OF STRUCTURAL COMPONENTS.
- VERIFY AND COORDINATE, WITH ALL CONTRACTORS, THE LOCATION OF ALL ARCHITECTURAL AND MECHANICAL APERTURANCES AND OPENINGS.
- PROTECTION TO BE PROVIDED AT ALL PIPE LOCATIONS PER TYPICAL DETAIL ON SHEET S521.
- REFER TO PLUMBING DRAWINGS FOR ROOF DRAIN LOCATIONS.
- EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ.
- ADHESIVE ANCHORS SHALL BE HILTI HIT-HY 200.
- SLEEVE ANCHORS SHALL BE HILTI HLC.
- SCREW ANCHORS SHALL BE HILTI KWIK HUS.
- CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR THE FOLLOWING ITEMS PRIOR TO FABRICATION: CONCRETE REINFORCING, POST-TENSIONING, STRUCTURAL STEEL.

POST-TENSIONING

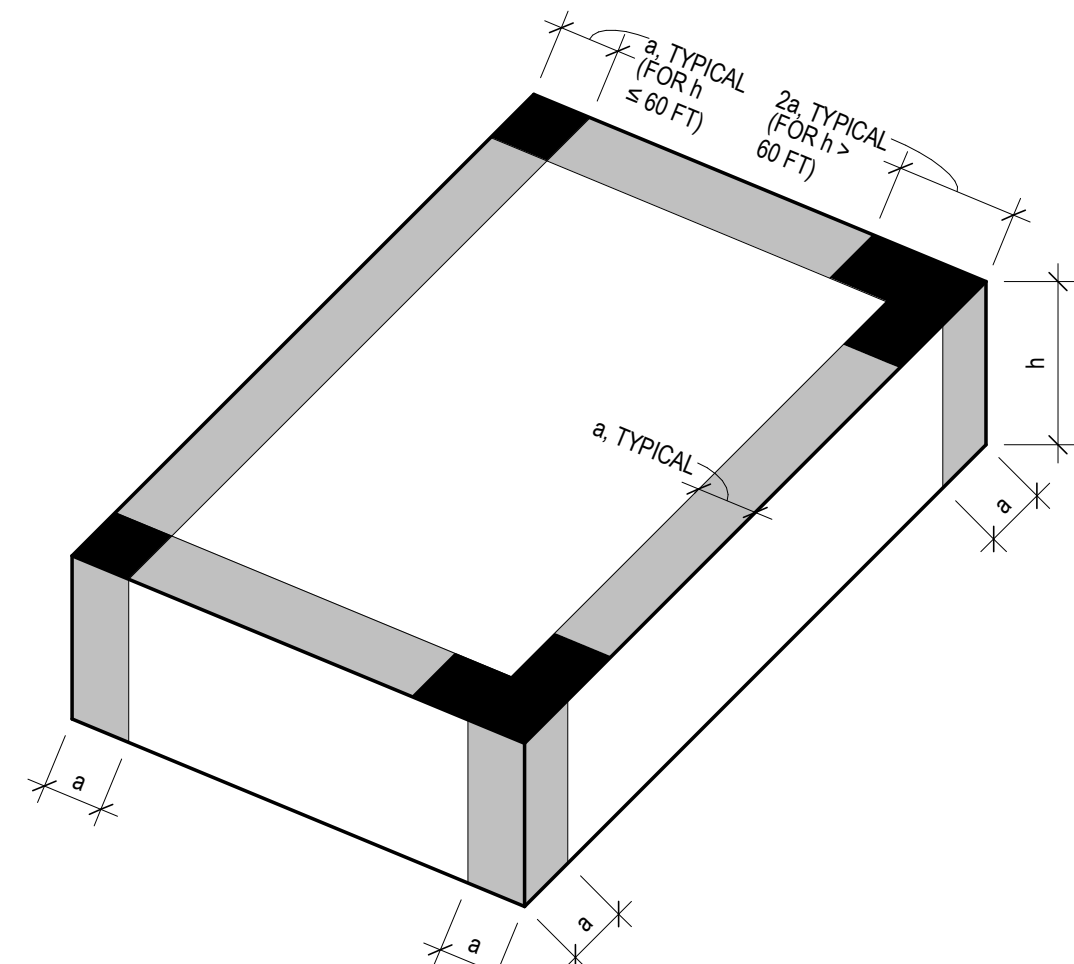
- POST-TENSIONING CABLES SHALL BE LOW RELAXATION 7-WIRE STRAND WITH ULTIMATE TENSILE CAPACITY OF 270 KSI.
- ACTUAL LOSSES SHALL BE COMPUTED BY TENDON SUPPLIER AND FURNISHED TO THE ENGINEER FOR RECORD PURPOSES ONLY.
- DEAD STRESSING ENDS IN A SLAB SHALL BE ANCHORED AT MID-DEPTH.
- TENDONS SHALL BE DRAPED ON A PARABOLIC PROFILE WITH LOW POINTS AND HIGH POINTS AS DETAILED ON THE DRAWINGS. TENDON PLACEMENT SHALL NOT VARY VERTICALLY BY MORE THAN 1/8-INCH.
- PARTIAL STRANDS NEEDED IN ADDITION TO FULL LENGTH STRANDS SHALL BE ANCHORED AT THE 1/4 POINT OF THE ADJACENT SPAN.
- PROVIDE MINIMUM OF TWO #4 BARS BEHIND ALL POST-TENSIONING ANCHORAGES. BARS SHALL BE CONTINUOUS FOR SLABS AND AS DETAILED FOR BEAMS.
- A MINIMUM OF THREE 1/2-INCH DIAMETER OR TWO 3/8-INCH DIAMETER CABLES SHALL PASS THROUGH ALL COLUMNS IN EACH DIRECTION FOR SLAB SYSTEM CONSTRUCTION.
- ALL ANCHORAGE HARDWARE SHALL MEET THE REQUIREMENTS OF PCI AND ACI. ANCHORS SHALL BE DESIGNED FOR 3750 PSI CONCRETE STRENGTH. SUBMIT ALL CALCULATIONS TO THE ENGINEER.
- THE GENERAL CONTRACTOR SHALL COORDINATE THE FINAL LOCATIONS OF CONSTRUCTION JOINTS WITH THE POST-TENSIONING SUPPLIER AND TO SUBMIT POURING AND STRESSING SEQUENCE TO THE ENGINEER DURING POST-TENSIONING SHOP DRAWING SUBMITTALS.
- MAXIMUM LENGTH OF TENDONS WHICH CAN BE PULLED FROM ONE END TO BE 125 FEET. JACKING FROM BOTH ENDS SHALL BE PERFORMED WHEN THERE IS EXCESSIVE FRICTIONAL LOSS ANTICIPATED. POST-TENSIONING SUPPLIER TO SUBMIT ALL CALCULATIONS SHOWING POST TENSION LOSSES TO THE ENGINEER.
- INSERTS AND FASTENING DEVICES DUE TO OTHER WORK: INSERTS MAY BE USED ONLY IN AREAS WHERE THERE WILL BE NO INTERFERENCE WITH POST-TENSIONING TENDONS AND/OR ANCHORAGES. IN NO CASE MAY EMBEDDED ITEMS BE ATTACHED TO POST-TENSIONING STEEL, AND CARE SHALL BE TAKEN SO AS NOT TO HAVE THE TENDONS OUT OF THEIR DESIGNED POSITIONS. POWDER-DRIVEN OR DRILLED-IN INSERTS WILL NOT BE PERMITTED, UNLESS OTHERWISE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. AT A MINIMUM, POST TENSIONING TENDONS NEED TO BE LOCATED PRIOR TO DRILLING IN POST-TENSIONED CONCRETE SLABS, BEAMS, OR GIRDERS.

CONCRETE COVER REQUIREMENTS FOR POST-TENSIONED TENDONS

COMPONENT	REQUIRED COVER
UNRESTRAINED SLABS (BOTTOM)	1 1/2"
UNRESTRAINED SLABS (TOP)	2"

- ALL TENDONS TO BE ENCAPSULATED PER ACI 302 AND PTI SPECIFICATIONS.

WIND PROVISIONS FOR COMPONENTS AND CLADDING



FLAT ROOF BUILDING

<div></div> = INTERIOR ZONE ROOF = ZONE 1 WALLS = ZONE 4	<div></div> = END ZONE ROOF = ZONE 2 WALLS = ZONE 5	<div></div> = CORNER ZONE ROOF = ZONE 3
--	---	--

ZONE	DESIGN WIND PRESSURE, PSF							DESCRIPTION
	EFFECTIVE WALL AREA, SF							
	1	10	20	50	100	200	> 500	
1	53	53	53	53	53	53	53	ROOF INTERIOR ZONE
2	59.4	59.4	58.5	57.2	56.2	56.2	56.2	END ZONE REGION OF THE ROOF
3	75.5	75.5	69.7	62	56.2	56.2	56.2	CORNER ZONE REGION OF THE ROOF
4 (+)	46.6	46.6	45.1	43	41.5	40	37.9	WALL INTERIOR ZONE
4 (-)	-49.5	-49.5	-47.9	-45.9	-44.4	-42.8	-40.8	
5 (+)	46.6	46.6	45.1	43	41.5	40	37.9	END ZONE REGION OF THE WALL
5 (-)	-58.2	-58.2	-55.5	-51	-47.9	-44.9	-44.8	

NOTES:

- NEGATIVE WIND PRESSURES ACT AWAY FROM COMPONENT SURFACE. POSITIVE WIND PRESSURES ACT TOWARD COMPONENT SURFACES.

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PROJECT TITLE:

VILLAGE ON PARK PARKING
STRUCTURE AND SITE
IMPROVEMENTS

808 HUGHES PLACE
MADISON, WI 53713

ISSUE:

07/28/2023 BID DOCUMENTS

PROJECT INFORMATION:

PROJECT NUMBER: 20225013.00

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DRAWN BY: JL

CHECKED BY: AMG

APPROVED BY: DFW

SCALE: AS NOTED

SHEET TITLE:

GENERAL NOTES

SHEET NUMBER:

S001

SYMBOL LEGEND

LINE	MATERIAL	REFERENCE
BEYOND	ALUMINUM	NORTH INDICATOR
CENTER, GRID	BRICK	DETAIL VIEW INDICATOR
DEMOLITION	CONCRETE	ELEVATION VIEW INDICATOR
EXISTING (HALF-TONE)	CONCRETE MASONRY BLOCK	SECTION VIEW INDICATOR
HIDDEN	EARTH	VIEW INDICATOR
MATCHLINE	GRAVEL	GRID INDICATOR
NEW (CUT)	GROUT	EXISTING GRID INDICATOR
NEW (PROJECTION)	WOOD STRUCTURAL PANEL	
OVERHEAD	STEEL	
		BREAK LINE
		SPAN DIRECTION INDICATOR
		SLOPE INDICATOR
		STEP INDICATOR
		OPENING (FLOOR, ROOF OR WALL)
		ELEVATION INDICATOR
		KEYNOTE INDICATOR
		REVISION CLOUD
		REVISION INDICATOR

VIEW LOCATION LEGEND

D1	D2	D3	D4	D5	D6
C1	C2	C3	C4	C5	C6
B1	B2	B3	B4	B5	B6
A1	A2	A3	A4	A5	A6

SHEET INDEX

S001	GENERAL NOTES
S002	GENERAL INFORMATION
S003	GENERAL FOUNDATION DETAILS
S004	GENERAL MASONRY DETAILS
S101	LEVEL ONE FOUNDATION PLAN
S102	LEVEL TWO FRAMING PLAN
S103	LEVEL THREE FRAMING PLAN
S104	LEVEL FOUR FRAMING PLAN
S105	LEVEL FIVE FRAMING PLAN
S106	LEVEL SIX FRAMING PLAN
S107	ROOF FRAMING PLAN
S108	EXISTING BUILDING FOUNDATION & ROOF PLAN
S201	BUILDING SECTIONS
S400	ENLARGED FLOOR PLANS
S401	ENLARGED FLOOR PLANS
S402	ENLARGED FLOOR PLANS
S403	ENLARGED FLOOR PLANS
S501	FOUNDATION DETAILS
S502	FOUNDATION DETAILS
S511	POST-TENSIONING DETAILS
S512	POST-TENSIONING DETAILS
S521	STRUCTURAL DETAILS
S522	STRUCTURAL DETAILS
S523	STRUCTURAL DETAILS
S524	STRUCTURAL DETAILS
S601	SCHEDULES
S602	MISCELLANEOUS SCHEDULES

STEEL SHAPES LEGEND

SECTION	TOP (T)	FRONT (F)	BACK (B)	BOTTOM (BOT)	LEGEND
T BOT					BG, DLH, G, K, LH, VG
T BOT					C, MC
T BOT					L
T BOT					(2) L
T BOT					HP, M, W
T BOT					HSS
T BOT					HSS (ROUND), PIPE
T BOT					MT, WT

STEEL LEGEND

	STEEL ROOF DECK (LONGITUDINAL)
	STEEL ROOF DECK (TRANSVERSE)
	BOLT
	THREADED ROD
	SHEAR STUD

CONCRETE LEGEND

	CONCRETE SLAB ON GRADE (LONGITUDINAL)
	CONCRETE SLAB ON STEEL DECK (LONGITUDINAL)
	CONCRETE SLAB ON STEEL DECK (TRANSVERSE)
	WELDED WIRE FABRIC REINFORCEMENT
	SHEAR STUDRAIL REINFORCEMENT
	ANCHOR ROD
	ADHESIVE ANCHOR
	EXPANSION ANCHOR
	SCREW ANCHOR
	BENT BAR 2'-0" H x 3'-0" V
	BAR DRILLED AND EPOXY SET INTO CONCRETE

CONCRETE TWO-WAY FLAT SLAB FRAMING PLAN LEGEND

	UNIFORM POST-TENSIONING
	REINFORCEMENT, SHOWN AND SCHEDULED ON PLAN
	FLOOR TYPE MARK
	COLUMN STRIP OR MIDDLE STRIP BOUNDARY
	BANDED POST-TENSIONING
	SHEARHEAD MARK
	BEAM MARK
	OPENING
	COLUMN
	EDGE OF SLAB
	EMBEDDED PLATE

FOUNDATION PLAN LEGEND

	SPREAD FOOTING
	HELICAL PILE
	GRADE BEAM
	CONCRETE WALL
	FOOTING MARK
	TOP OF FOUNDATION ELEVATION
	CONCRETE COLUMN OR PIER MARK
	SLAB ON GRADE CONSTRUCTION JOINT

ABBREVIATIONS

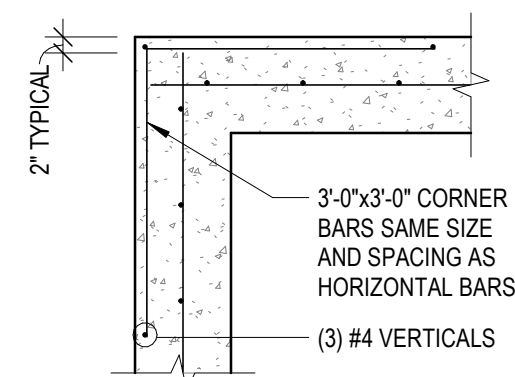
1WAY	ONE-WAY	LD BRG	LOAD-BEARING
AB	ANCHOR BOLT	LDV	LONG DIMENSION VERTICAL
ADDL	ADDITIONAL	LL	LONG DIMENSION HORIZONTAL
ADDM	ADDENDUM	LLB	LONG LEG BACK TO BACK
AHU	AIR HANDLING UNIT	LLH	LONG LEG HORIZONTAL
ALT	ALTERNATE	LLV	LONG LEG VERTICAL
APPROX	APPROXIMATE	LVL	LAMINATED VENEER LUMBER
ARCH	ARCHITECT	LVR	LOUVER
B PL	BASE PLATE	M	MOMENT
B/B	BACK TO BACK	MAX	MAXIMUM
BC	BOTTOM CHORD	MBR	MEMBER
BF	BOTH FACES	MC	MOMENT CONNECTION
BF	BOTTOM FACE	MD	METAL DECK
BLDG	BUILDING	MECH	MECHANICAL
BM	BEAM	MEZZ	MEZZANINE
BO	BOTTOM OF (REFER TO TOP OF _)	MFR	MANUFACTURER
BO	BOTTOM	MFR REC	MANUFACTURER'S RECOMMENDATION
BRD	BRIDGING	MD	MIDDLE
BRG	BEARING	MIN	MINIMUM
BRG PL	BEARING PLATE	MISC	MISCELLANEOUS
BS	BOTH SIDES	ML	MONOLITHIC
BSMT	BASEMENT	MO	MASONRY OPENING
BW	BOTH WAYS	MTL	METAL
		MULT	MULTIPLE
C	CHANNEL	N	NORTH
C TO C	CENTER TO CENTER	NF	NEAR FACE
CANTIL	CANTILEVER	NIC	NOT IN CONTRACT
CIP	CAST-IN-PLACE	NLB	NON LOAD-BEARING
CJ	CONSTRUCTION JOINT	NO	NUMBER
CJ	CONTROL JOINT	NOM	NOMINAL
CL	CENTER LINE	NS	NEAR SIDE
CMU	CONCRETE MASONRY UNIT	NTS	NOT TO SCALE
COL	COLUMN	OC	ON CENTER
CONC	CONCRETE	OD	OUTSIDE DIAMETER
CONN	CONNECT	OF	OUTSIDE FACE
CONSTR	CONSTRUCTION	OPNG	OPENING
CONT	CONTINUOUS	OPH	OPPOSITE HAND
CONTR	CONTRACTOR	OPP	OPPOSITE
CRPS	COMPRESSIBLE	OPT	OPTIONAL
CTRL	CONTROL	O/O	OUT TO OUT
CU	CUBIC	PCC	PRECAST CONCRETE
CU FT	CUBIC FEET	PCF	POUNDS PER CUBIC FOOT
CU IN	CUBIC INCH	PL	PLATE
CU YD	CUBIC YARD	PLF	POUNDS PER LINEAR FOOT
D	DEEP	PLYWD	PLYWOOD
D	DEPTH	PREST	PRECAST
DBE	DECK BEARING ELEVATION	PRELIM	PRELIMINARY
DBL	DOUBLE	PS CONC	PRESTRESSED CONCRETE
DEG	DEGREE	PSF	POUNDS PER SQUARE FOOT
DET	DETAIL	PSI	POUNDS PER SQUARE INCH
DEMO	DEMOLITION	PT	POST-TENSIONED
DIA	DIAMETER	PT CONC	PRESSURE TREATED POST-TENSIONED CONCRETE
DIM	DIMENSION		
DIR	DIRECTION	QTY	QUANTITY
DWG	DRAWING	R	RADIUS
E	EAST	RD	ROOF DRAIN
EA	EACH	REF	REFERENCE
EE	EACH END	REIN	REINFORCE
EF	EACH FACE	REDO	REQUIRED
EJ	EXPANSION JOINT	REV	REVISION
EL	ELEVATION	RS	ROUGH SAWN
ELEC	ELECTRIC	RTU	ROOF TOP UNIT
ELEV	ELEVATOR	S	SOUTH
ENGR	ENGINEER	SCHED	SCHEDULE
EOD	EDGE OF DECK	SCHEM	SCHEMATIC
EOS	EDGE OF GRATING	SE	STRUCTURAL ENGINEER
EQ	EQUAL	SECT	SECTION
EQ SP	EQUALLY SPACED	SF	SQUARE FOOT (FEET)
EQUIP	EQUIPMENT	SHT	SHEET
EQUIV	EQUIVALENT	SIM	SIMILAR
EW	EACH WAY	SL	SLAB
EXC	EXCAVATE	SLBB	SHORT LEG BACK TO BACK
EXIST (E)	EXISTING	SP	SUMP PIT
EXP	EXPANSION	SPCL	SPECIAL
EXP BT	EXPANSION BOLT	SPEC	SPECIFICATION
EXT	EXTERIOR	SO	SQUARE
FD	FLOOR DRAIN	SO IN	SQUARE INCH
FDTN	FOUNDATION	SO YD	SQUARE YARD
FF	FAR FACE	STD	STANDARD
FLR	FLOOR	STF	STIFFENER
FR	FRAME	STL_JST	STEEL JOIST
FS	FAR SIDE	STRUC	STRUCTURAL
FSNR	FASTENER	SYMM	SYMMETRICAL
FT	FEET	T&B	TOP AND BOTTOM
FTG	FOOTING	TB	THROUGH BOLT
FUT	FUTURE	TC	TOP CHORD
GA	GAGE	TEMJ	TEMP ELEVATION MATCHES JOIST
GALV	GALVANIZED	TEMP	TEMPORARY
GC	GENERAL CONTRACTOR	THK	THICKNESS
GLU	GLUE	THRU	THROUGH
GLU LAM	GLUE LAMINATED WOOD	TOB	TOP OF BEAM
GR BM	GRADE BEAM	TOC	TOP OF CONCRETE
GRGT	GRATING	TOCF	TOP OF COLD-FORMED
		TOD	TOP OF DECK
H	HIGH	TOPE	TOP OF DRILLED PIER
HDR	HEADER	TOF	TOP OF FOUNDATION
HGR	HANGER	TOG	TOP OF GRATING
HORIZ	HORIZONTAL	TOGB	TOP OF GRADE BEAM
HS	HIGH STRENGTH	TOP	TOP OF PIER
HSPKG	HOUSEKEEPING	TOPC	TOP OF PILE CAP
HT	HEIGHT	TOS	TOP OF STEEL
I	MOMENT OF INERTIA	TOW	TOP OF WALL
ID	INSIDE DIAMETER	TS	TUBE STEEL
IF	INSIDE FACE	TYP	TYPICAL
INFO	INFORMATION	UNO	UNLESS NOTED OTHERWISE
INT	INTERIOR	VAR	VARIES
JST	JOIST	VERT	VERTICAL
JST BRG	JOIST BEARING	VIF	VERIFY IN FIELD
K	KIP (ONE THOUSAND POUNDS)	W	WEST
KB	KNEE BRACE	WI	WITH
KLF	KIPS PER LINEAR FOOT	WIO	WITHOUT
KOP	KNOCK OUT PANEL	WBL	WOOD BLOCKING
KSF	KIPS PER SQUARE FOOT	WD	WOOD
KSI	KIPS PER SQUARE INCH	WF	WIDE FLANGE
KWY	KEYWAY	WP	WORK POINT
L	ANGLE	WT	WEIGHT
LATL	LATERAL	WWF	WELDED WIRE FABRIC
LB	POUND	WWM	WELDED WIRE MESH
		YD	YARD

PRECAST CONCRETE LEGEND

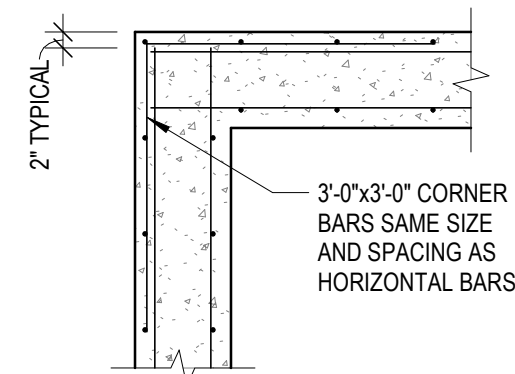
	HOLLOW CORE PRECAST PLANK (LONGITUDINAL)
	HOLLOW CORE PRECAST PLANK (TRANSVERSE)
	PRECAST TEE (LONGITUDINAL)
	PRECAST TEE (TRANSVERSE)



Autodesk Docs://20225013.00 - Village on Park Garage/20225013_S022.rvt
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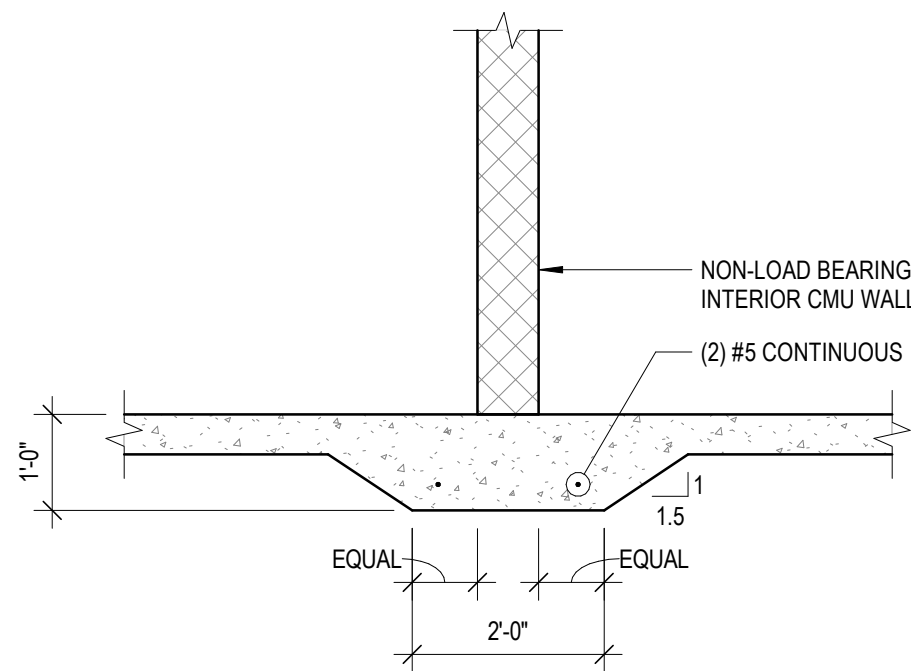
ONE LAYER OF REINFORCEMENT



TWO LAYERS OF REINFORCEMENT

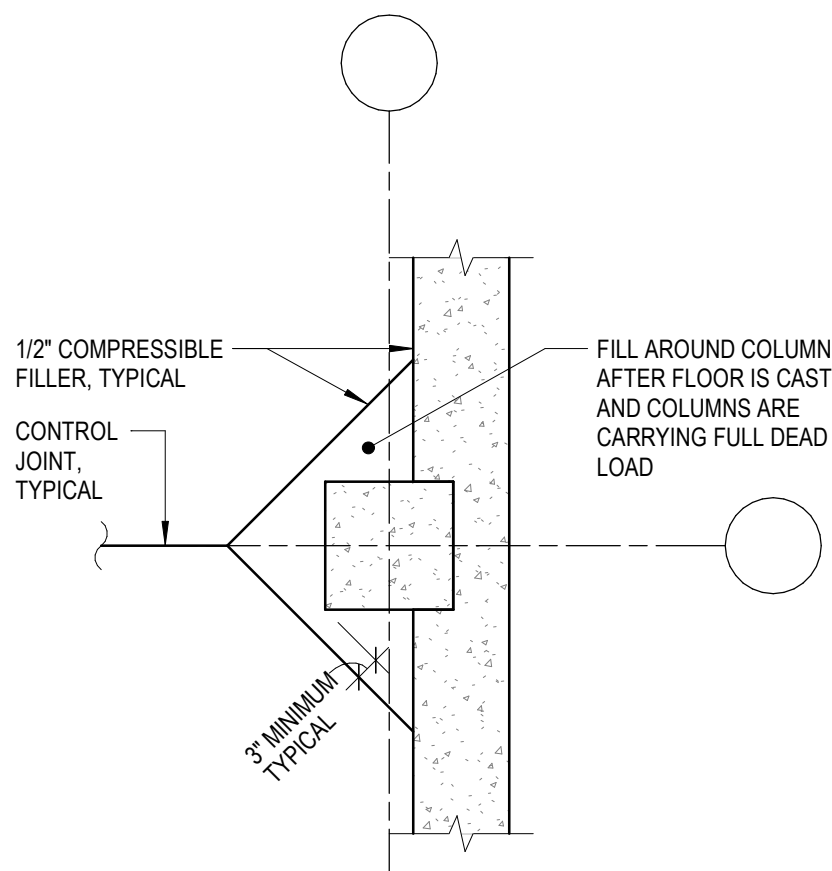
D5 WALL CORNER

1/2" = 1'-0"



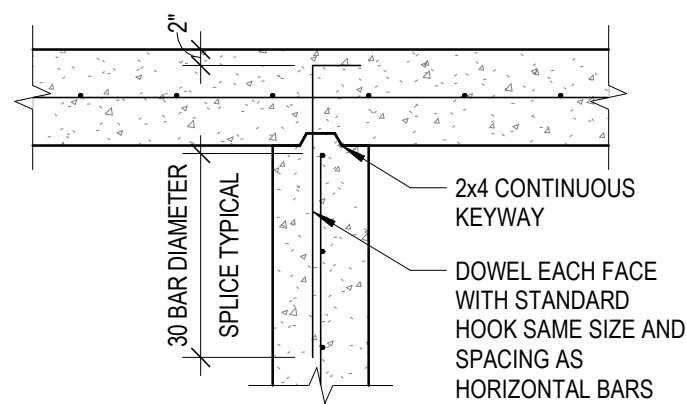
D6 CMU WALL ON SLAB ON GRADE

1/2" = 1'-0"

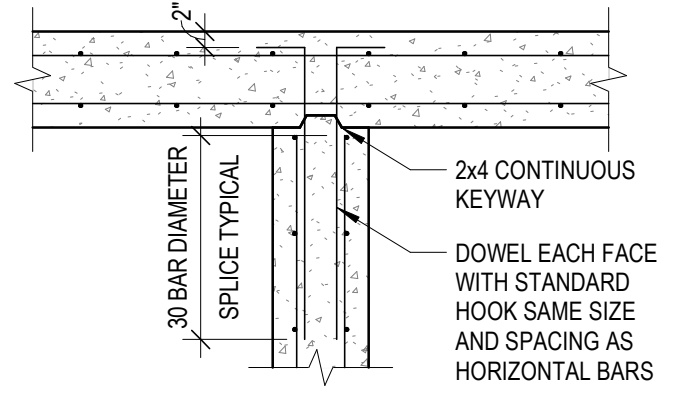


C4 EXTERIOR COLUMN ISOLATION JOINT

1/2" = 1'-0"



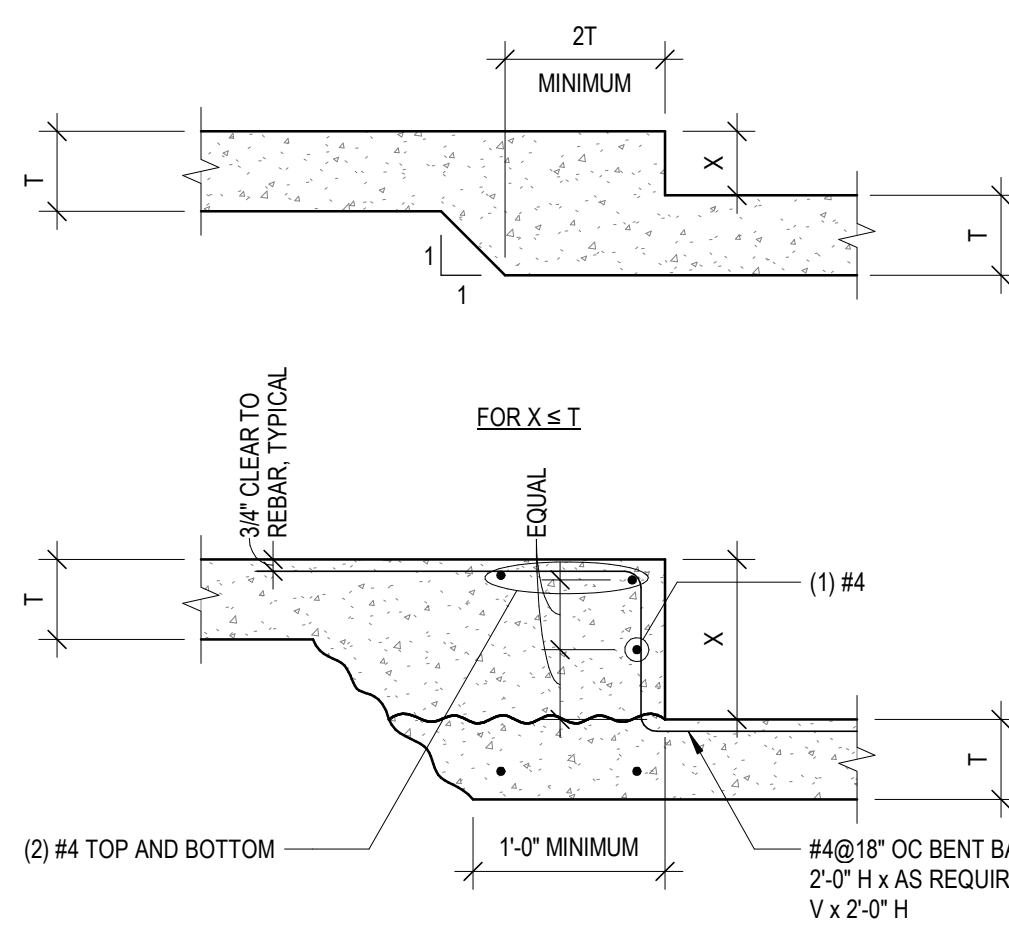
ONE LAYER OF REINFORCEMENT



TWO LAYERS OF REINFORCEMENT

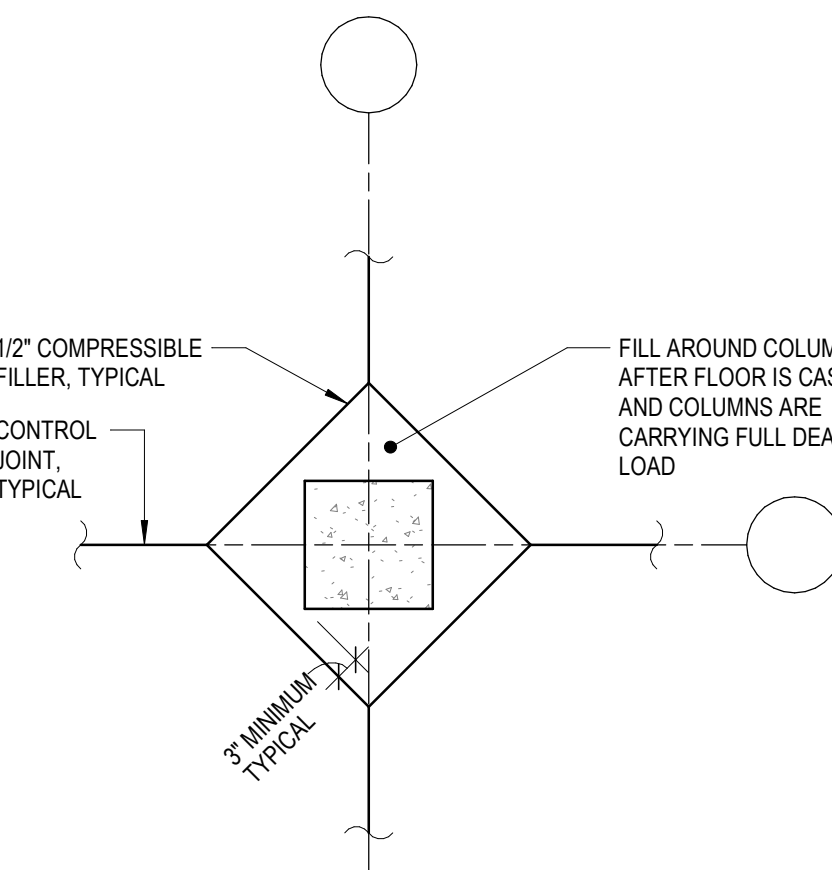
C5 WALL INTERSECTION

1/2" = 1'-0"



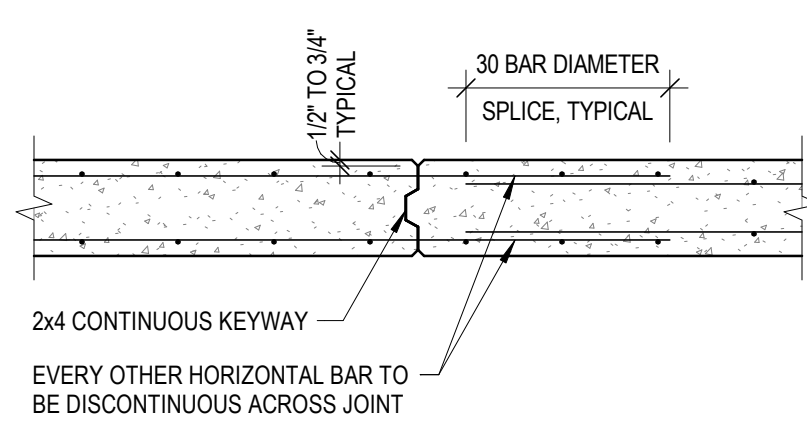
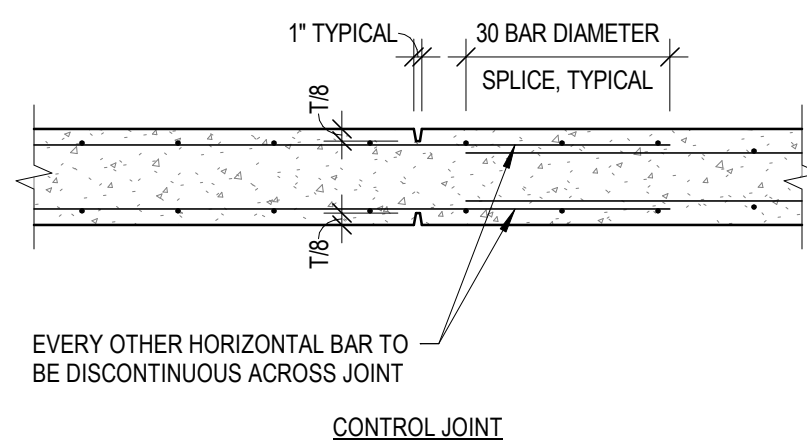
C6 SLAB ON GRADE DEPRESSION

1" = 1'-0"



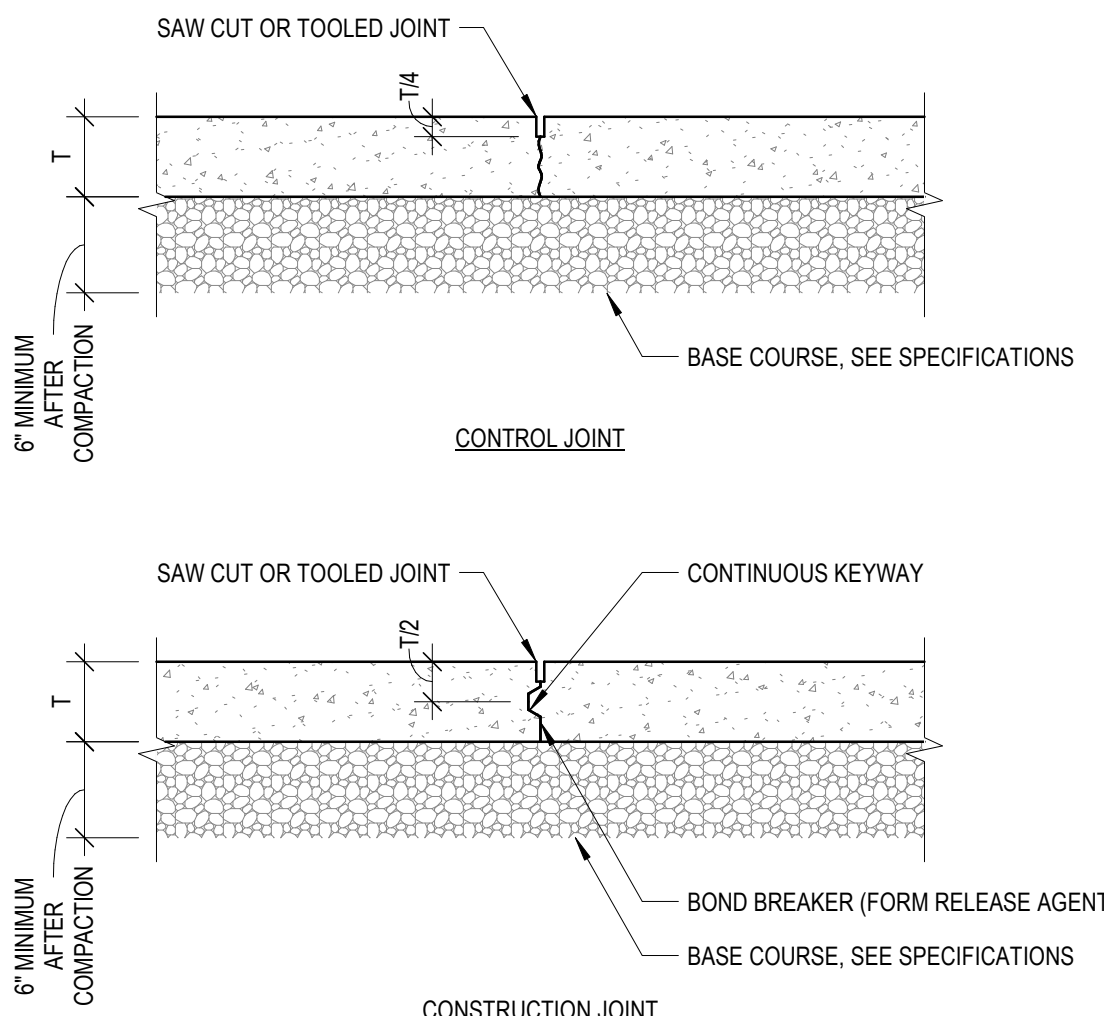
B4 INTERIOR COLUMN ISOLATION JOINT

1/2" = 1'-0"



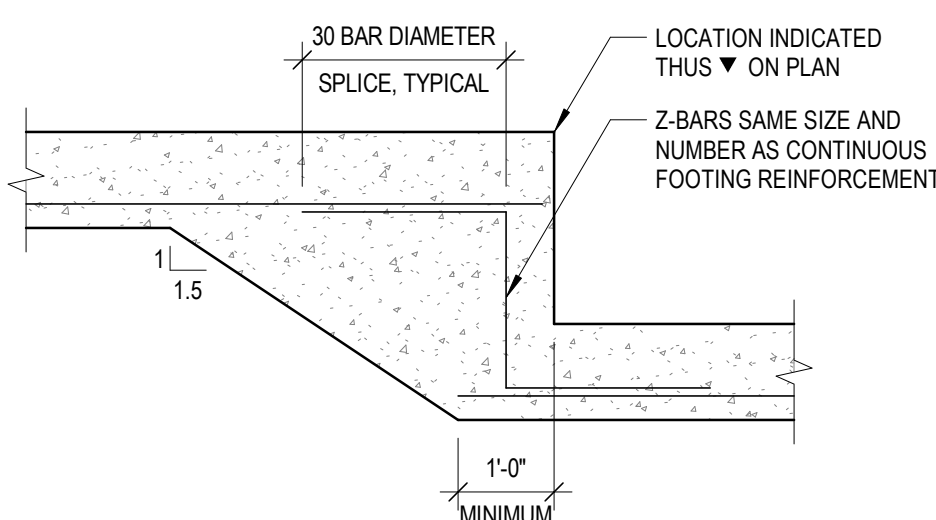
B5 WALL CONTROL AND CONSTRUCTION JOINT

1/2" = 1'-0"



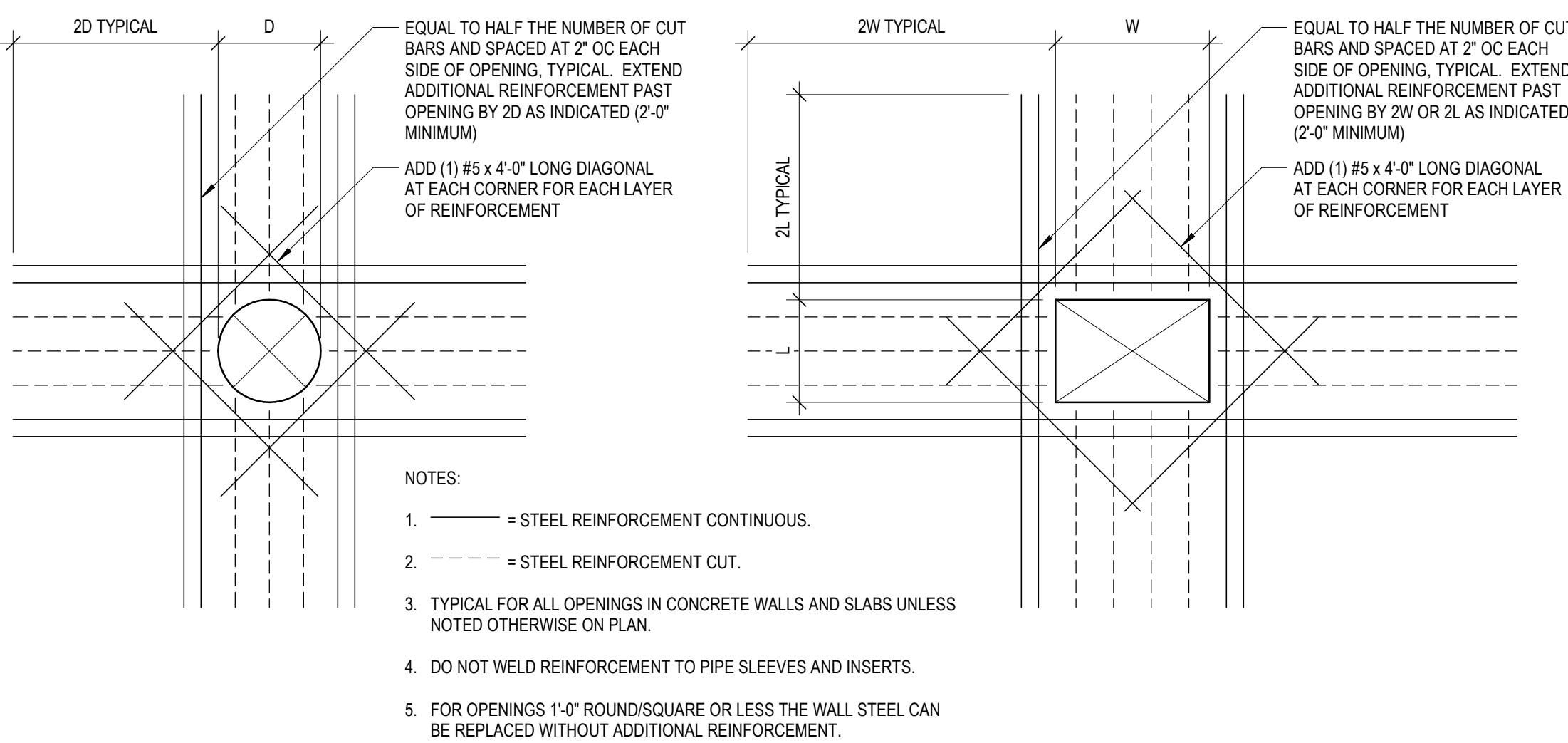
B6 SLAB ON GRADE CONTROL AND CONSTRUCTION JOINT

1" = 1'-0"



A4 FOOTING STEP

1/2" = 1'-0"



A5 OPENING REINFORCEMENT

1/2" = 1'-0"

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808 HUGHES PLACE
MADISON, WI 53713

ISSUE:

07/28/2023 BID DOCUMENTS

PROJECT INFORMATION:

PROJECT NUMBER: 20225013.00

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DRAWN BY: JL

CHECKED BY: AMG

APPROVED BY: DFW

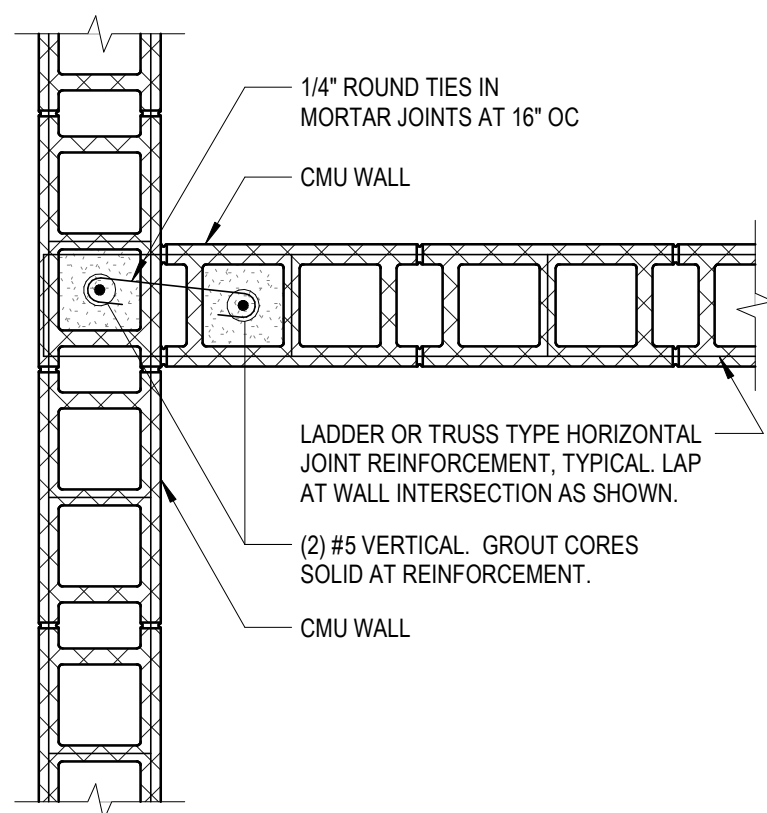
SCALE: AS NOTED

SHEET TITLE:

GENERAL MASONRY DETAILS

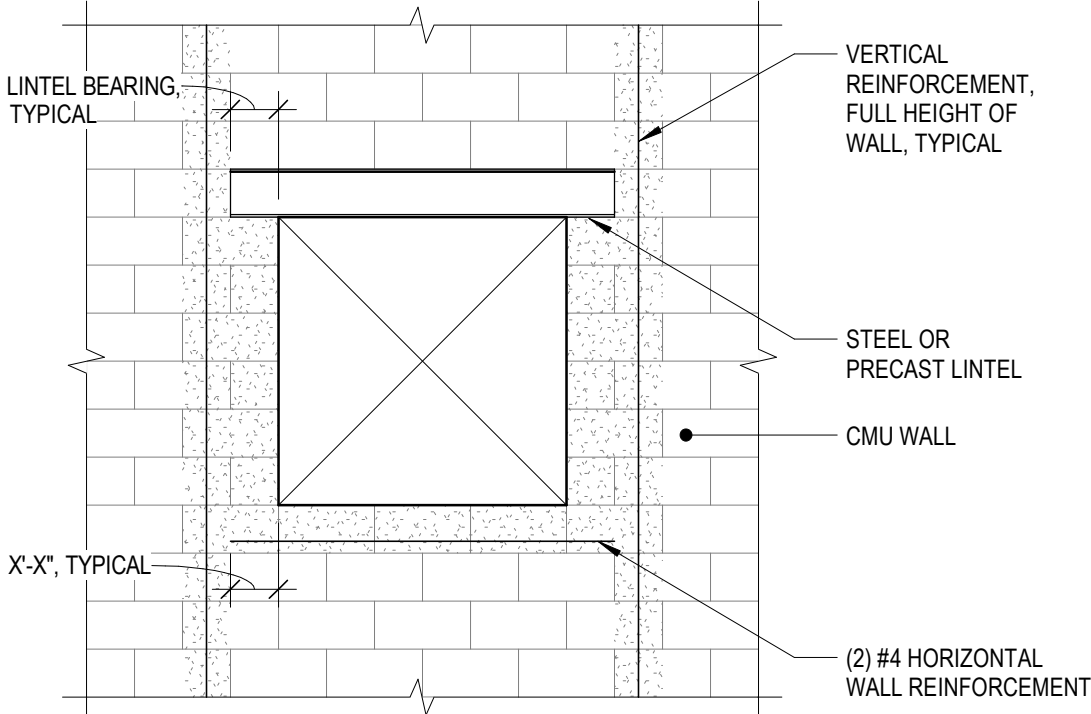
SHEET NUMBER:

S004



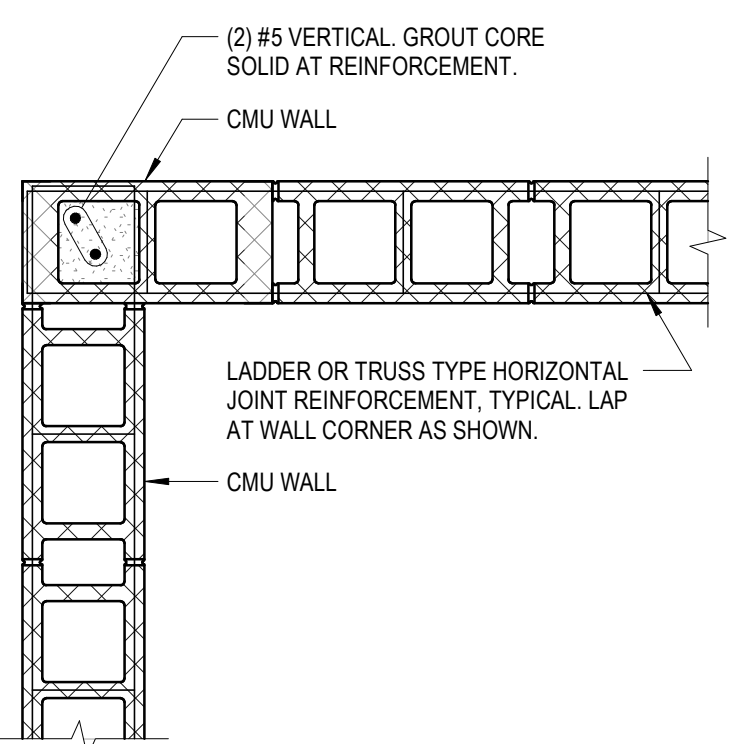
D5 CMU WALL INTERSECTION

1" = 1'-0"



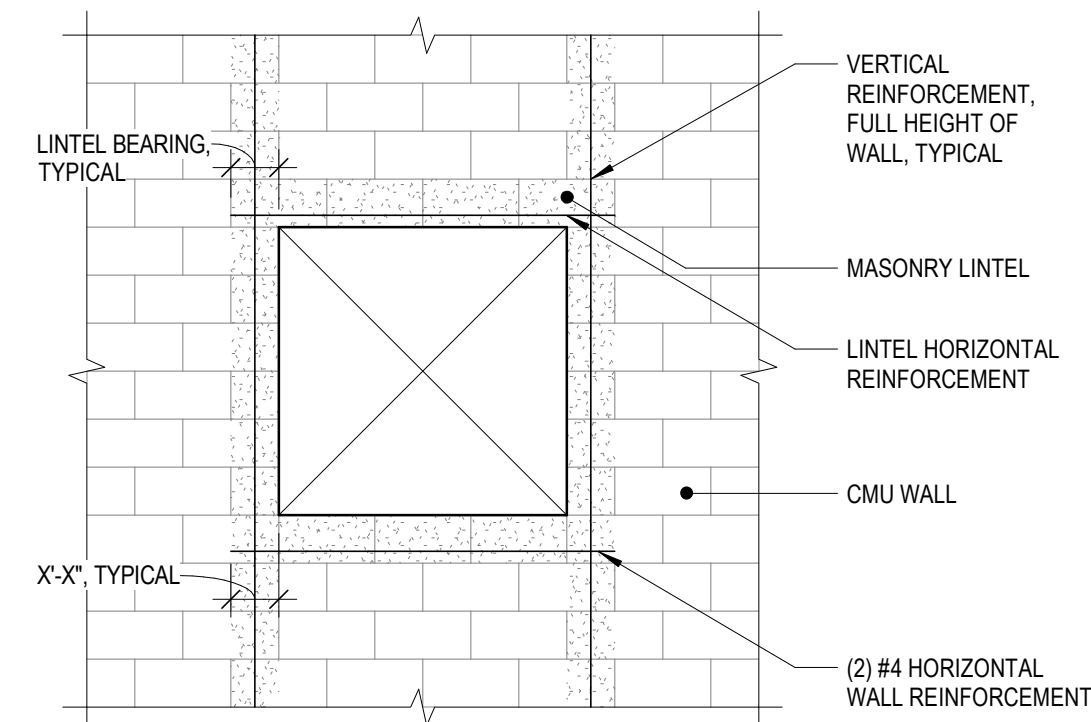
D6 CMU WALL PUNCHED OPENING

3/8" = 1'-0"



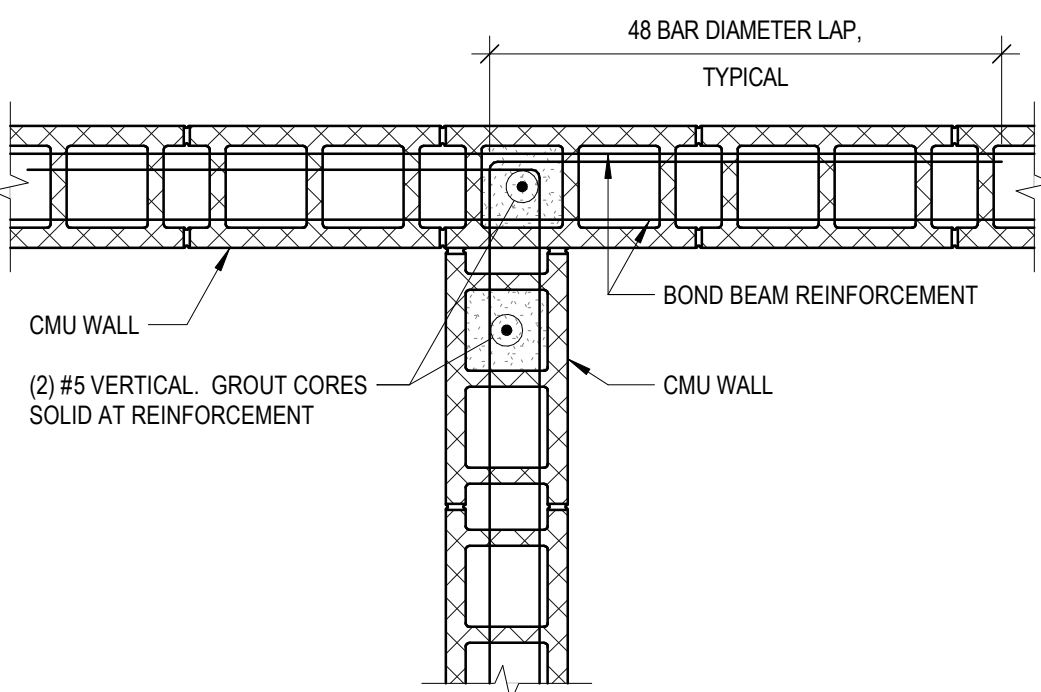
C5 CMU WALL CORNER

1" = 1'-0"



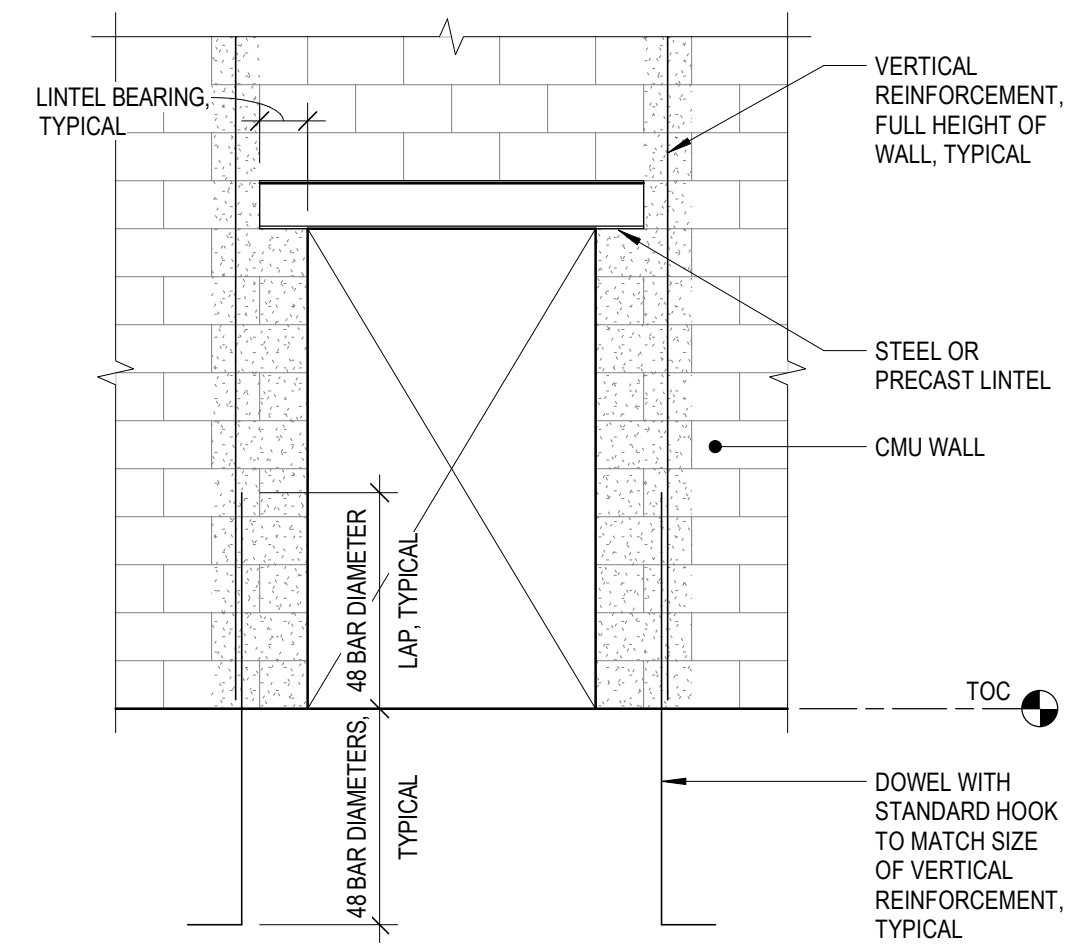
C6 CMU WALL PUNCHED OPENING

3/8" = 1'-0"



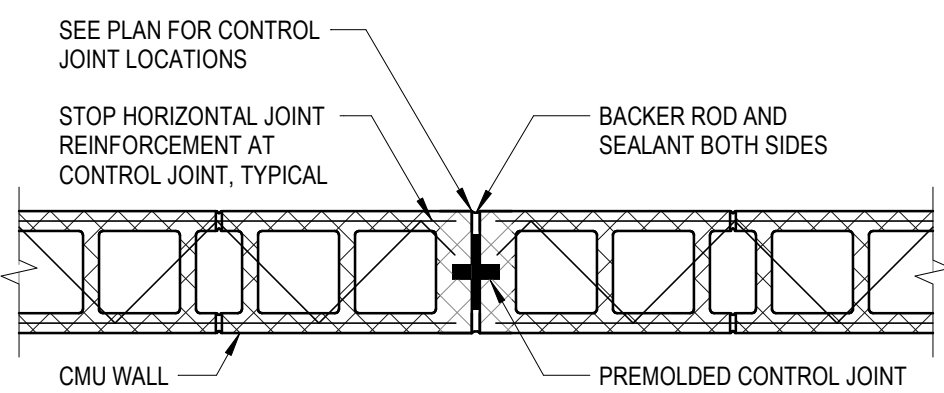
B5 BOND BEAM INTERSECTION

1" = 1'-0"



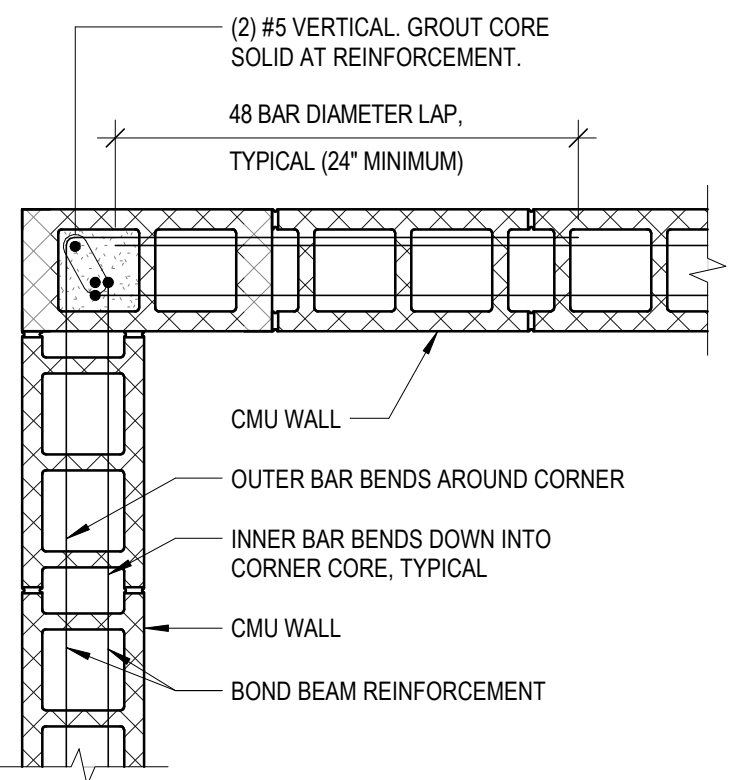
B6 CMU WALL DOOR OPENING

3/8" = 1'-0"



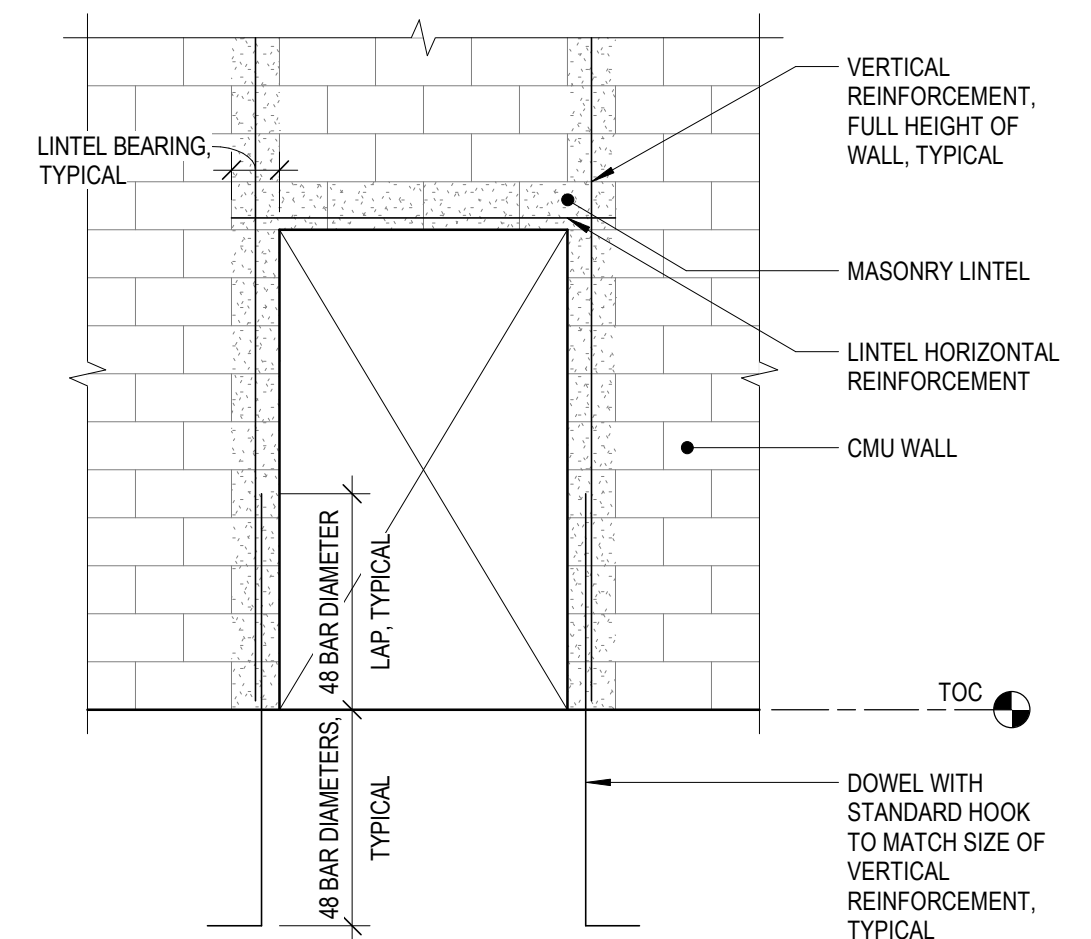
A4 CMU WALL VERTICAL CONTROL JOINT

1" = 1'-0"



A5 BOND BEAM CORNER

1" = 1'-0"



A6 CMU WALL DOOR OPENING

3/8" = 1'-0"



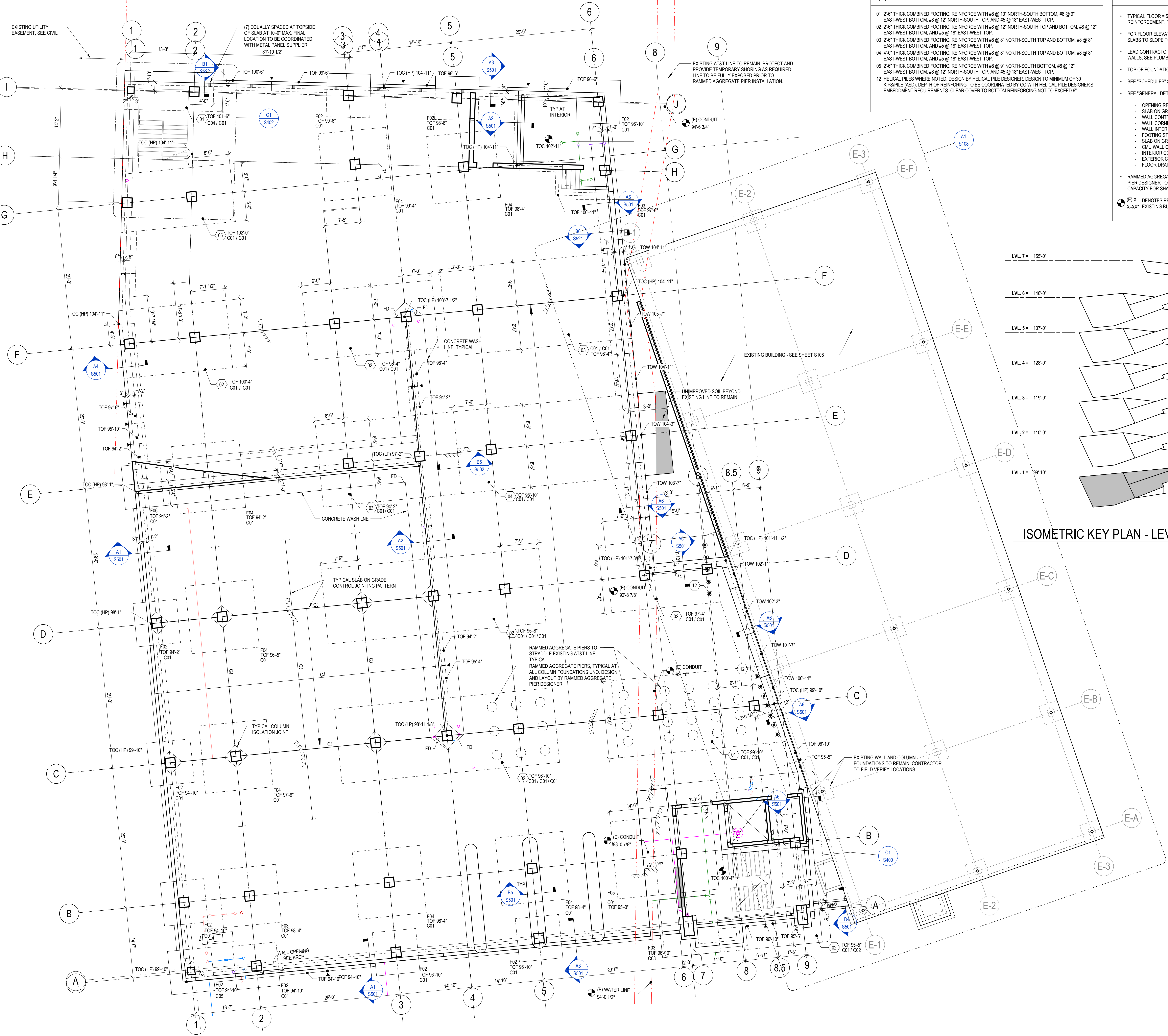
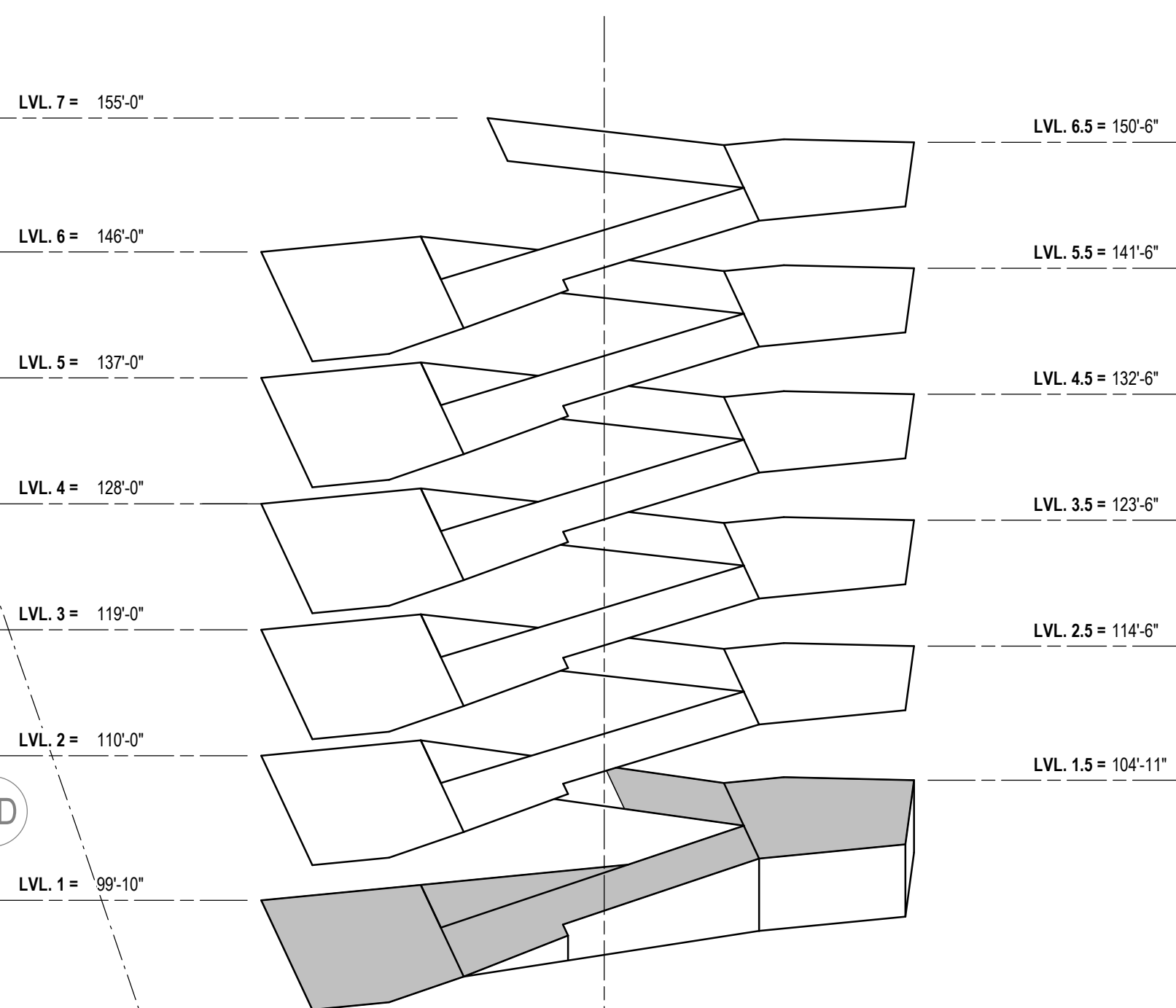
SHEET KEYNOTES

- 2'-6" THICK COMBINED FOOTING. REINFORCE WITH #8 @ 10" NORTH-SOUTH BOTTOM, #8 @ 9" EAST-WEST BOTTOM, AND #5 @ 12" NORTH-SOUTH TOP, AND #5 @ 18" EAST-WEST TOP.
- 2'-6" THICK COMBINED FOOTING. REINFORCE WITH #8 @ 12" NORTH-SOUTH TOP AND BOTTOM, #8 @ 12" EAST-WEST BOTTOM, AND #5 @ 18" EAST-WEST TOP.
- 2'-6" THICK COMBINED FOOTING. REINFORCE WITH #8 @ 8" NORTH-SOUTH TOP AND BOTTOM, #8 @ 8" EAST-WEST BOTTOM, AND #5 @ 18" EAST-WEST TOP.
- 4'-0" THICK COMBINED FOOTING. REINFORCE WITH #8 @ 8" NORTH-SOUTH TOP AND BOTTOM, #8 @ 8" EAST-WEST BOTTOM, AND #5 @ 18" EAST-WEST TOP.
- 2'-6" THICK COMBINED FOOTING. REINFORCE WITH #8 @ 9" NORTH-SOUTH BOTTOM, #8 @ 12" EAST-WEST BOTTOM, AND #5 @ 12" NORTH-SOUTH TOP, AND #5 @ 18" EAST-WEST TOP.
- HELICAL PILES WHERE NOTED, DESIGN BY HELICAL PILE DESIGNER. DESIGN TO MINIMUM OF 30 KIPS/PILE (ASD). DEPTH OF REINFORCING TO BE COORDINATED BY GC WITH HELICAL PILE DESIGNER'S EMBEDMENT REQUIREMENTS. CLEAR COVER TO BOTTOM REINFORCING NOT TO EXCEED 6".

GENERAL SHEET NOTES

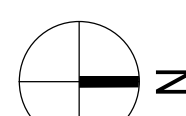
- TYPICAL FLOOR = 5" CONCRETE SLAB ON GRADE REINFORCED WITH 3.5 LB/CY YARD MACRO FIBER REINFORCEMENT. TOP OF CONCRETE SLAB ELEVATION VARIES. SEE PLAN.
- FOR FLOOR ELEVATIONS BETWEEN POINTS INDICATED USE STRAIGHT LINE INTERPOLATION. ALL SLABS TO SLOPE TO DRAIN, MAXIMUM OF 3/16" PER FOOT.
- LEAD CONTRACTOR TO COORDINATE DRAIN TILE LATERAL CONNECTIONS THROUGH FOUNDATION WALLS. SEE PLUMBING.
- TOP OF FOUNDATION WALL/GRADE BEAM NOTED TOW/OF X-X" ON PLAN.
- SEE "SCHEDULES" SHEET(S) FOR SPREAD FOOTINGS AND CONCRETE COLUMN SCHEDULES.
- SEE "GENERAL DETAILS" SHEET(S) FOR THE FOLLOWING DETAILS:
 - OPENING REINFORCEMENT
 - SLAB ON GRADE CONTROL AND CONSTRUCTION JOINT
 - WALL CONTROL AND CONSTRUCTION JOINT
 - WALL CORNER
 - WALL INTERSECTION
 - FOOTING STEP
 - SLAB ON GRADE DEPRESSION
 - CMU WALL ON SLAB ON GRADE
 - INTERIOR COLUMN ISOLATION JOINT
 - EXTERIOR COLUMN ISOLATION JOINT
 - FLOOR DRAIN AT SLAB ON GRADE
- RAMMED AGGREGATE PIERS TO BE INSTALLED AT ALL COLUMN FOUNDATIONS. RAMMED AGGREGATE PIER DESIGNER TO PROVIDE DESIGN AND LAYOUT FOR AN EQUIVALENT 5000 PSF SOIL BEARING CAPACITY FOR SHALLOW FOUNDATIONS.
- (E) X DENOTES RESULTS FROM JSD POTHOLING SURVEY OF APPROXIMATE TOP ELEVATIONS OF X-X" EXISTING BURIED AT&T 4" CONDUIT OR EXISTING WATER LINE.

ISOMETRIC KEY PLAN - LEVEL 1

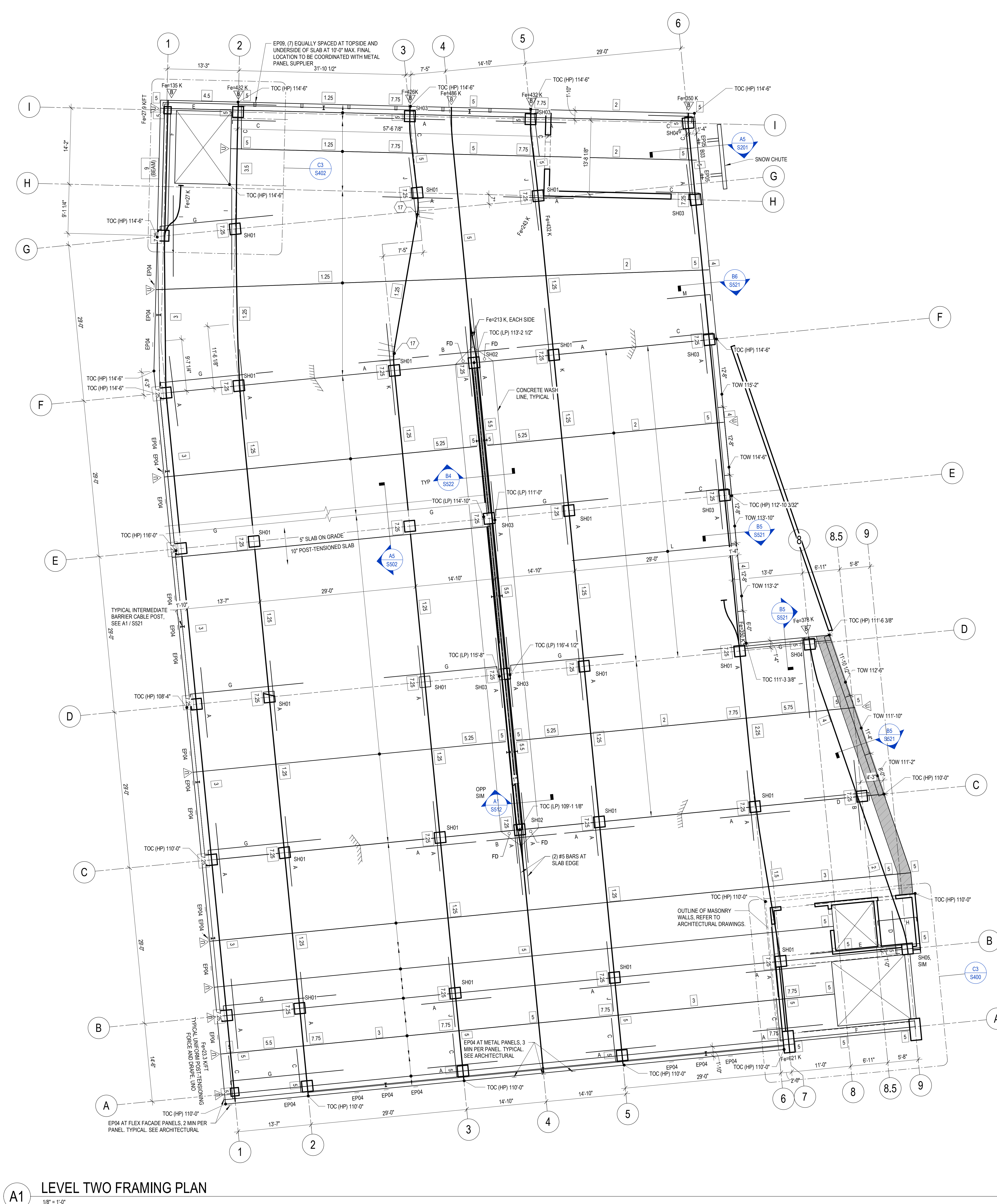


LEVEL ONE FOUNDATION PLAN

1/8" = 1'-0"



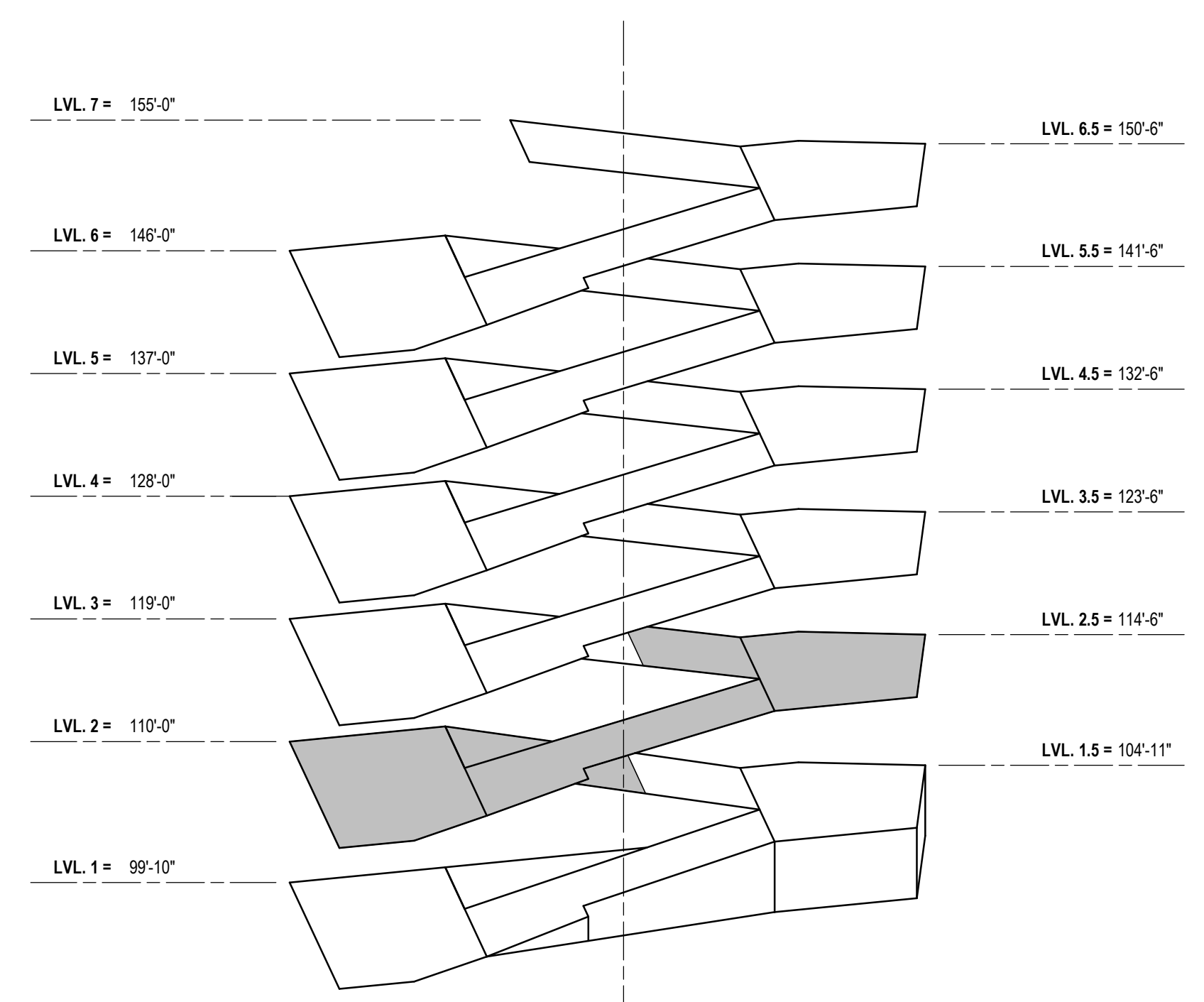
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A1 LEVEL TWO FRAMING PLAN
1/8" = 1'-0"

GENERAL SHEET NOTES

- TYPICAL FLOOR = 10' TWO-WAY FLAT WITH #4@16" OC EACH WAY BOTTOM UNLESS NOTED OTHERWISE. SEE S101 FOR SLAB ON GRADE NOTES. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE. TOP OF CONCRETE SLAB ELEVATION VARIES. SEE PLAN.
- FOR FLOOR ELEVATIONS BETWEEN POINTS INDICATED USE STRAIGHT LINE INTERPOLATION. ALL SLABS TO SLOPE TO DRAIN, MAXIMUM OF 3/16" PER FOOT.
- SEE "SCHEDULES" SHEET FOR CONCRETE BEAM AND SHEARHEAD SCHEDULES.
- MAINTAIN SPECIFIED SLAB THICKNESS THROUGHOUT FLAT SLAB UNLESS NOTED OTHERWISE.
- SEE ELECTRICAL, PLUMBING, FIRE PROTECTION, HVAC AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF SLAB OPENINGS AND PIPE SLEEVES. PROVIDE SLEEVES WITH DIAMETER 1" LARGER THAN CONDUIT. ALL SLEEVES TO BE SCHEDULE 40 STEEL PIPE.
- NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.
- SLAB REINFORCEMENT PLACING ORDER:
 - EW BOTTOM
 - NS BOTTOM
 - NS TOP
 - EW TOP
- REINFORCING NOTES (SEE PLAN FOR LOCATION):
 - A. 12 #5 x 13'-0" LONG (TOP)
 - B. 15 #5 x 13'-0" LONG (TOP)
 - C. 9 #5 x 8'-0" LONG (TOP), HOOKED
 - D. 10 #5 x 12'-0" LONG (TOP), HOOKED
 - E. 4 #5 x 21'-0" LONG (TOP), HOOKED
 - F. 8 #5 x 21'-0" LONG (TOP), HOOKED
 - G. 12 #5 x 21'-0" LONG (TOP), HOOKED
 - H. #5 @ 12" (TOP & BOTTOM)
 - I. 13 #5 x 15'-0" LONG (TOP)
 - J. 21 #5 x 13'-0" LONG (TOP)
 - K. 12 #5 x 17'-0" LONG (TOP)
 - L. #4 x 30'-0" @ 12" OC (BOTTOM), HOOKED
 - M. #4 x 5'-0" @ 16" OC (BOTTOM), HOOKED - TYPICAL AT SLAB INTERIOREXTERIOR PERIMETERS
 - N. 18 #5 x 13'-0" LONG (TOP)
 - O. 18 #5 x 21'-0" LONG TOP, HOOKED



ISOMETRIC KEY PLAN - LEVEL 2



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VILLAGE ON PARK PARKING
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808 HUGHES PLACE
MADISON, WI 53713

ISSUE:

07/28/2023 BID DOCUMENTS

PROJECT INFORMATION:

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DATE: 07/28/2023

DRAWN BY: JL

CHECKED BY: AMG

APPROVED BY: DFW

SCALE: AS NOTED

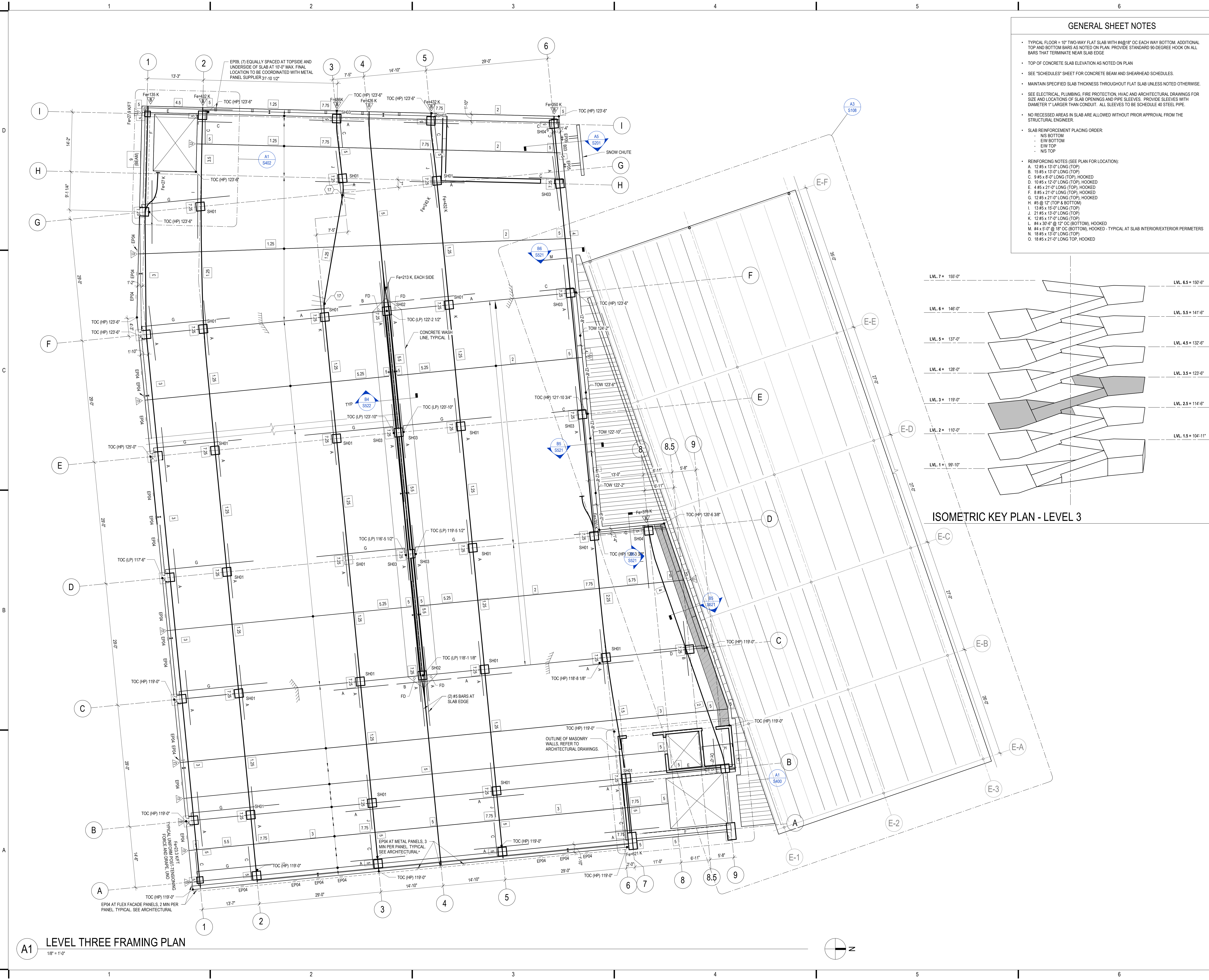
SHEET TITLE:

LEVEL TWO FRAMING PLAN

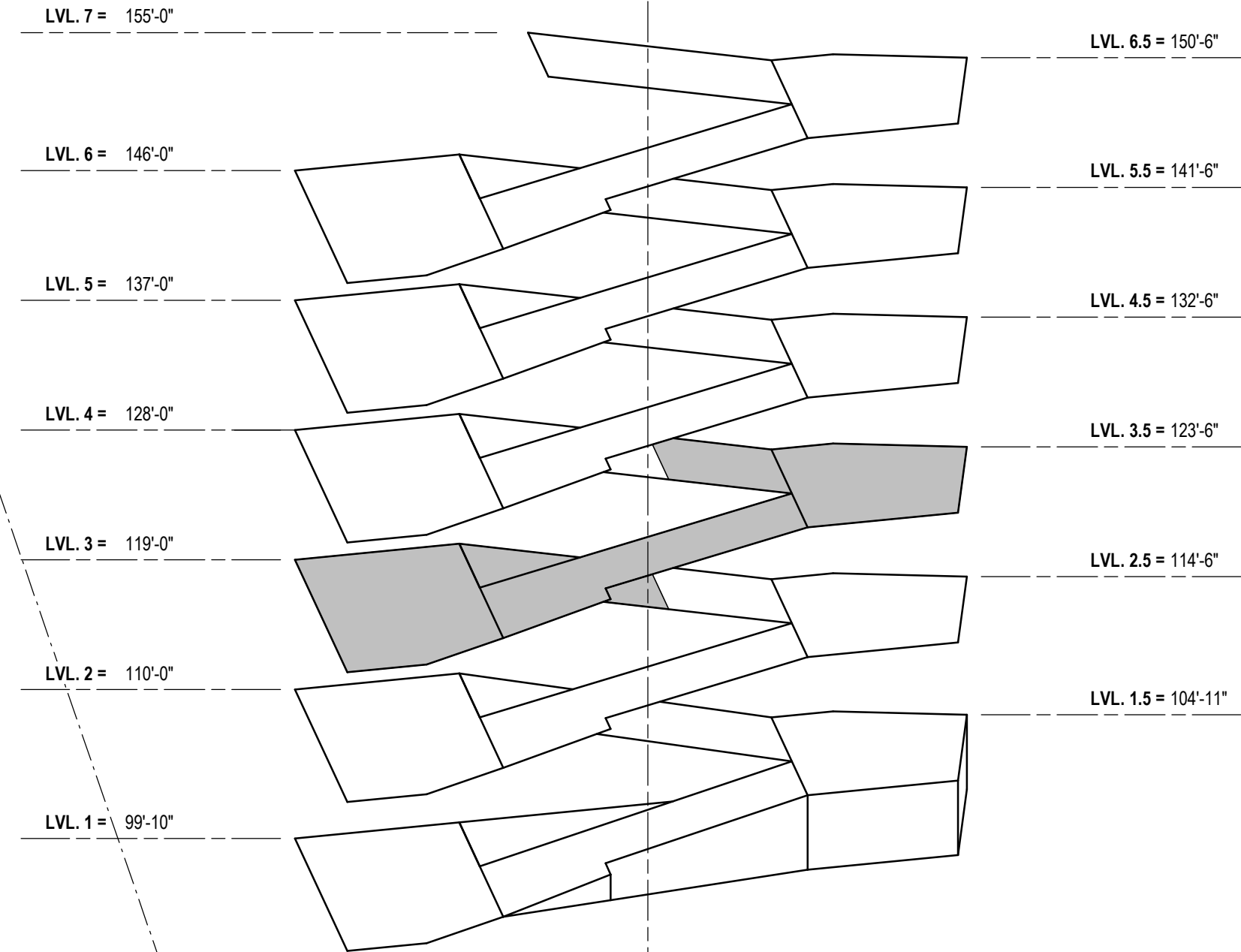
SHEET NUMBER:

S102

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- GENERAL SHEET NOTES**
- TYPICAL FLOOR = 10' TWO-WAY FLAT SLAB WITH #4@18" OC EACH WAY BOTTOM. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE
 - TOP OF CONCRETE SLAB ELEVATION AS NOTED ON PLAN
 - SEE "SCHEDULES" SHEET FOR CONCRETE BEAM AND SHEARHEAD SCHEDULES
 - MAINTAIN SPECIFIED SLAB THICKNESS THROUGHOUT FLAT SLAB UNLESS NOTED OTHERWISE
 - SEE ELECTRICAL, PLUMBING, FIRE PROTECTION, HVAC AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF SLAB OPENINGS AND PIPE SLEEVES. PROVIDE SLEEVES WITH DIAMETER 1" LARGER THAN CONDUIT. ALL SLEEVES TO BE SCHEDULE 40 STEEL PIPE.
 - NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER
 - SLAB REINFORCEMENT PLACING ORDER:
 - NS BOTTOM
 - EW BOTTOM
 - EW TOP
 - NS TOP
 - REINFORCING NOTES (SEE PLAN FOR LOCATION):
 - A. 12 #5 x 13'-0" LONG (TOP)
 - B. 15 #5 x 13'-0" LONG (TOP)
 - C. 9 #5 x 8'-0" LONG (TOP), HOOKED
 - D. 10 #5 x 12'-0" LONG (TOP), HOOKED
 - E. 4 #5 x 21'-0" LONG (TOP), HOOKED
 - F. 8 #5 x 21'-0" LONG (TOP), HOOKED
 - G. 12 #5 x 21'-0" LONG (TOP), HOOKED
 - H. #5 @ 12" (TOP & BOTTOM)
 - I. 13 #5 x 15'-0" LONG (TOP)
 - J. 21 #5 x 13'-0" LONG (TOP)
 - K. 12 #5 x 17'-0" LONG (TOP)
 - L. #4 x 30'-0" @ 12" OC (BOTTOM), HOOKED
 - M. #4 x 30'-0" @ 18" OC (BOTTOM), HOOKED - TYPICAL AT SLAB INTERIOR/EXTERIOR PERIMETERS
 - N. 18 #5 x 13'-0" LONG (TOP)
 - O. 18 #5 x 21'-0" LONG TOP, HOOKED



ISOMETRIC KEY PLAN - LEVEL 3

A1 LEVEL THREE FRAMING PLAN
1/8" = 1'-0"



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CHECKED BY: AMG

APPROVED BY: DFW

SCALE: AS NOTED

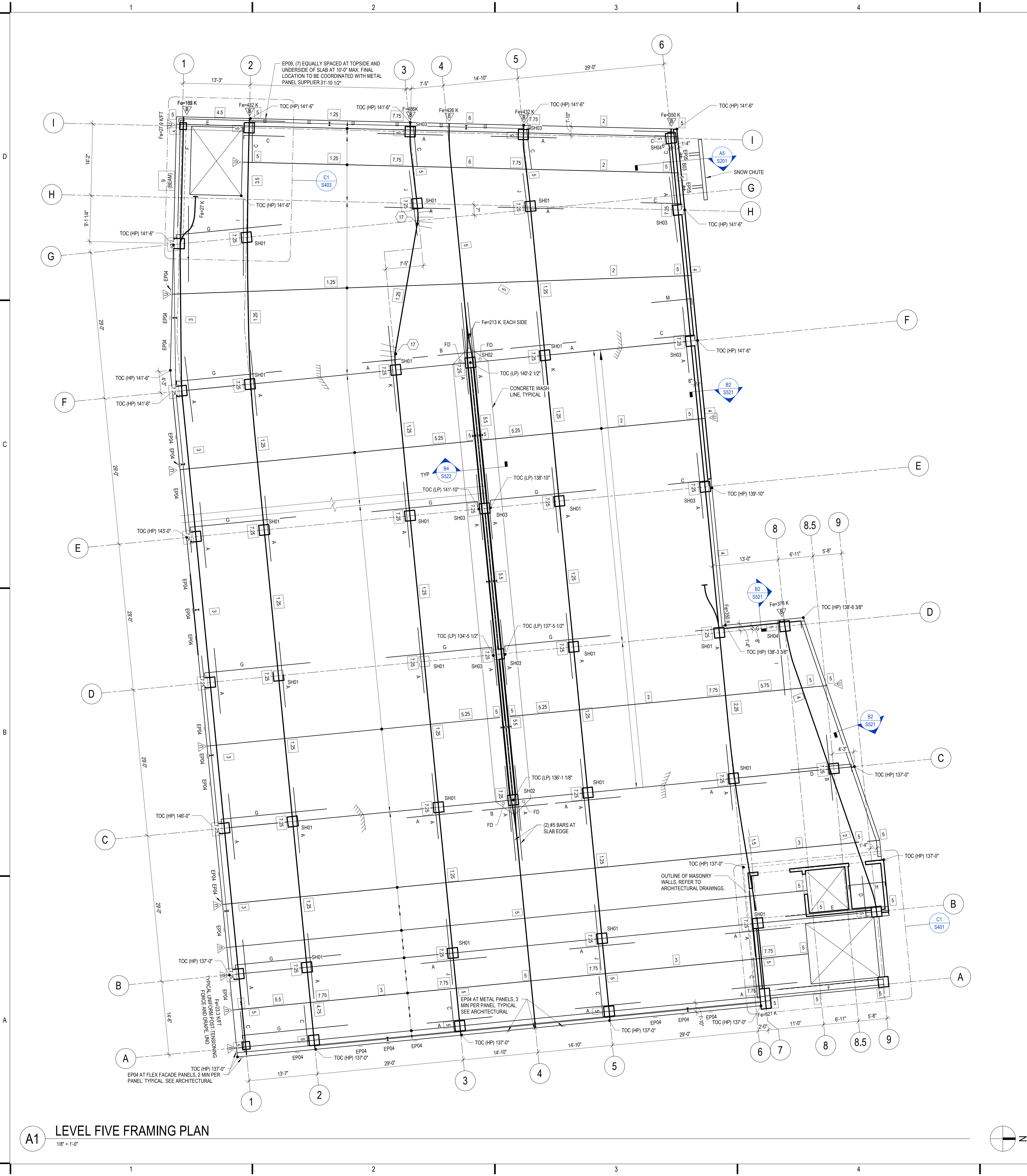
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LEVEL THREE FRAMING PLAN

SHEET NUMBER:

S103

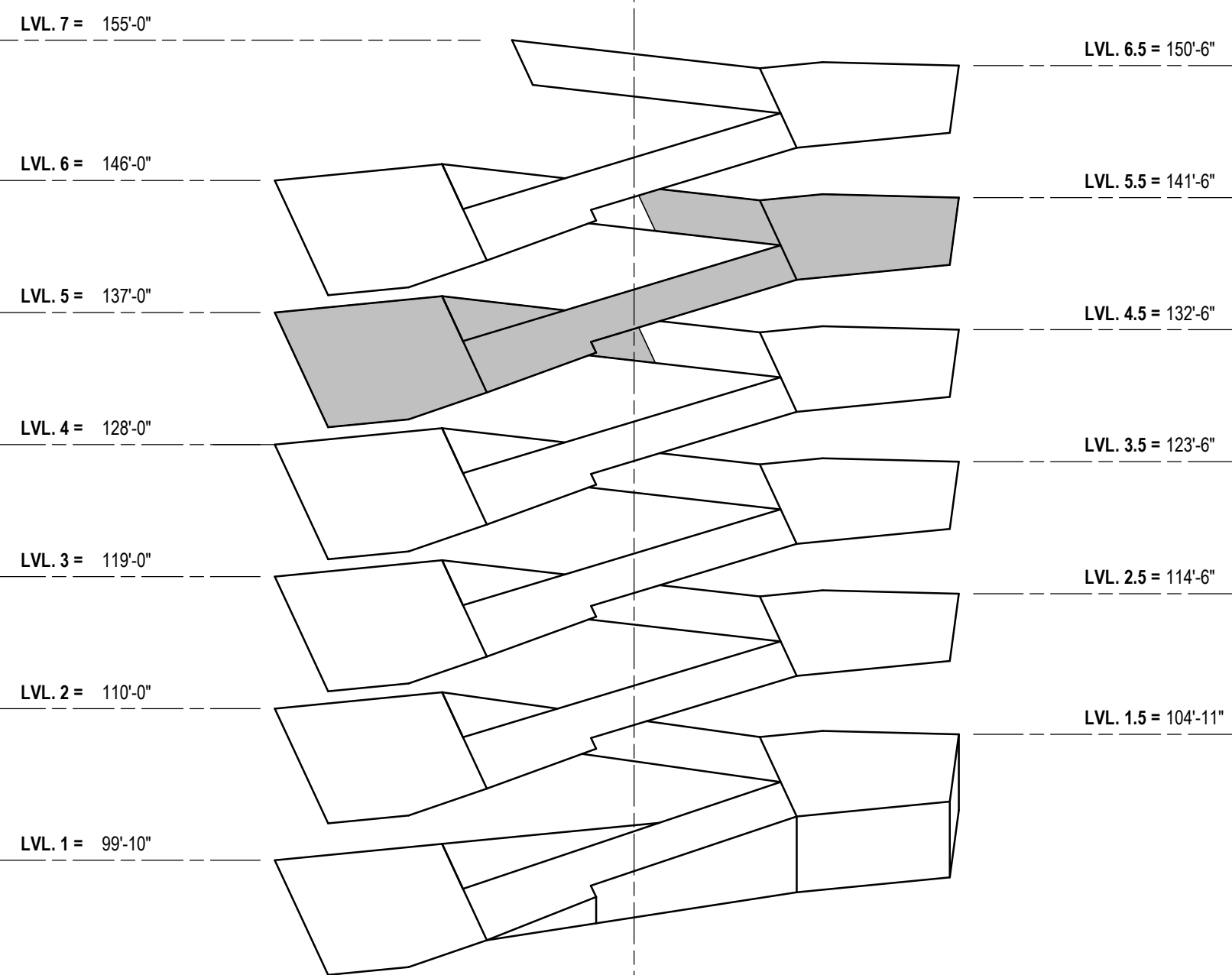
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A1 LEVEL FIVE FRAMING PLAN
1/8" = 1'-0"

GENERAL SHEET NOTES

- TYPICAL FLOOR = 10' TWO-WAY FLAT SLAB WITH #4@18" OC EACH WAY BOTTOM. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE
- TOP OF CONCRETE SLAB ELEVATION AS NOTED ON PLAN
- SEE "SCHEDULES" SHEET FOR CONCRETE BEAM AND SHEARHEAD SCHEDULES
- MAINTAIN SPECIFIED SLAB THICKNESS THROUGHOUT FLAT SLAB UNLESS NOTED OTHERWISE
- SEE ELECTRICAL, PLUMBING, FIRE PROTECTION, HVAC AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF SLAB OPENINGS AND PIPE SLEEVES. PROVIDE SLEEVES WITH DIAMETER 1" LARGER THAN CONDUIT. ALL SLEEVES TO BE SCHEDULE 40 STEEL PIPE.
- NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER
- SLAB REINFORCEMENT PLACING ORDER:
 - NS BOTTOM
 - EW BOTTOM
 - EW TOP
 - NS TOP
- REINFORCING NOTES (SEE PLAN FOR LOCATION):
 - A. 12 #5 x 13'-0" LONG (TOP)
 - B. 15 #5 x 13'-0" LONG (TOP)
 - C. 9 #5 x 8'-0" LONG (TOP), HOOKED
 - D. 10 #5 x 12'-0" LONG (TOP), HOOKED
 - E. 4 #5 x 21'-0" LONG (TOP), HOOKED
 - F. 8 #5 x 21'-0" LONG (TOP), HOOKED
 - G. 12 #5 x 21'-0" LONG (TOP), HOOKED
 - H. #5 @ 12" (TOP & BOTTOM)
 - I. 13 #5 x 15'-0" LONG (TOP)
 - J. 21 #5 x 13'-0" LONG (TOP)
 - K. 12 #5 x 17'-0" LONG (TOP)
 - L. #4 x 30'-0" @ 12" OC (BOTTOM), HOOKED
 - M. #4 x 30'-0" @ 18" OC (BOTTOM), HOOKED - TYPICAL AT SLAB INTERIOR/EXTERIOR PERIMETERS
 - N. 18 #5 x 13'-0" LONG (TOP)
 - O. 18 #5 x 21'-0" LONG (TOP), HOOKED



ISOMETRIC KEY PLAN - LEVEL 5

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SHEET TITLE:

LEVEL FIVE FRAMING PLAN

SHEET NUMBER:

S105

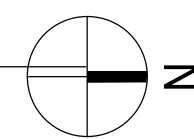
$$1/8'' = 1'-0''$$

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A1

ROOF FRAMING PLAN

1/8" = 1'-0"



PV SYSTEM COLUMN LOADING SCHEDULE

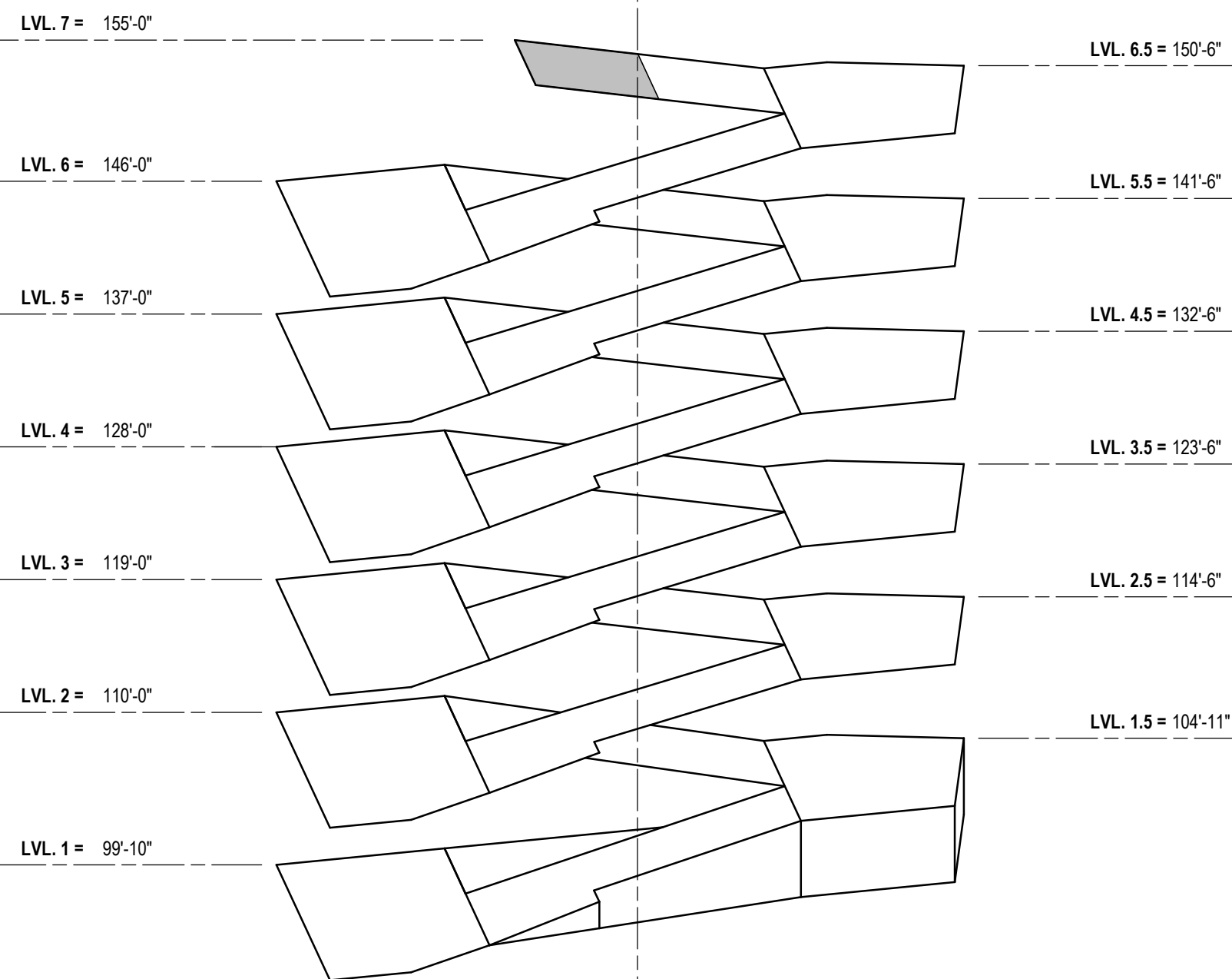
MARK	DEAD (GRAVITY)	DEAD (LATERAL)	SNOW (GRAVITY)	SNOW (LATERAL)	WIND: NS (GRAVITY)	WIND: NS (LATERAL)	WIND: E/W (GRAVITY)	WIND: E/W (LATERAL)
R _{L1}	5 KIPS	2 KIPS NS	13 KIPS	7 KIPS NS	28 KIPS	15 KIPS	31 KIPS	12 KIPS
R _{L1}	5 KIPS	8 KIPS NS	24 KIPS	24 KIPS NS	55 KIPS	56 KIPS	52 KIPS	52 KIPS

NOTES:

- DEAD AND SNOW LOADS GIVEN AT SERVICE LEVEL.
- WIND LOADS GIVEN AT ULTIMATE AND ARE REVERSIBLE.
- LATERAL LOADING DIRECTION GIVEN RELATIVE TO TRUE NORTH.

GENERAL SHEET NOTES

- TYPICAL FLOOR = 10' TWO-WAY FLAT SLAB WITH #4@18" OC EACH WAY BOTTOM. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE.
- TOP OF CONCRETE SLAB ELEVATION AS NOTED ON PLAN.
- SEE "SCHEDULES" SHEET FOR CONCRETE BEAM AND SHEARHEAD SCHEDULES.
- MAINTAIN SPECIFIED SLAB THICKNESS THROUGHOUT FLAT SLAB UNLESS NOTED OTHERWISE.
- SEE ELECTRICAL, PLUMBING, FIRE PROTECTION, HVAC AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF SLAB OPENINGS AND PIPE SLEEVES. PROVIDE SLEEVES WITH DIAMETER 1" LARGER THAN CONDUIT. ALL SLEEVES TO BE SCHEDULE 40 STEEL PIPE.
- NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.
- SLAB REINFORCEMENT PLACING ORDER:
 - NS BOTTOM
 - EW BOTTOM
 - EW TOP
 - NS TOP
- REINFORCING NOTES (SEE PLAN FOR LOCATION):
 - A. 12 #5 x 13'-0" LONG (TOP)
 - B. 15 #5 x 13'-0" LONG (TOP)
 - C. 9 #5 x 8'-0" LONG (TOP), HOOKED
 - D. 10 #5 x 12'-0" LONG (TOP), HOOKED
 - E. 4 #5 x 21'-0" LONG (TOP), HOOKED
 - F. 8 #5 x 21'-0" LONG (TOP), HOOKED
 - G. 12 #5 x 21'-0" LONG (TOP), HOOKED
 - H. #5 @ 12" (TOP & BOTTOM)
 - I. 13 #5 x 15'-0" LONG (TOP)
 - J. 21 #5 x 13'-0" LONG (TOP)
 - K. 12 #5 x 17'-0" LONG (TOP)
 - L. #4 x 30'-0" @ 12" OC (BOTTOM), HOOKED
 - M. #4 x 30'-0" @ 18" OC (BOTTOM), HOOKED - TYPICAL AT SLAB INTERIOR/EXTERIOR PERIMETERS
 - N. 18 #5 x 13'-0" LONG (TOP)
 - O. 18 #5 x 21'-0" LONG (TOP), HOOKED



ISOMETRIC KEY PLAN - ROOF

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ROOF FRAMING PLAN

SHEET NUMBER:

S107

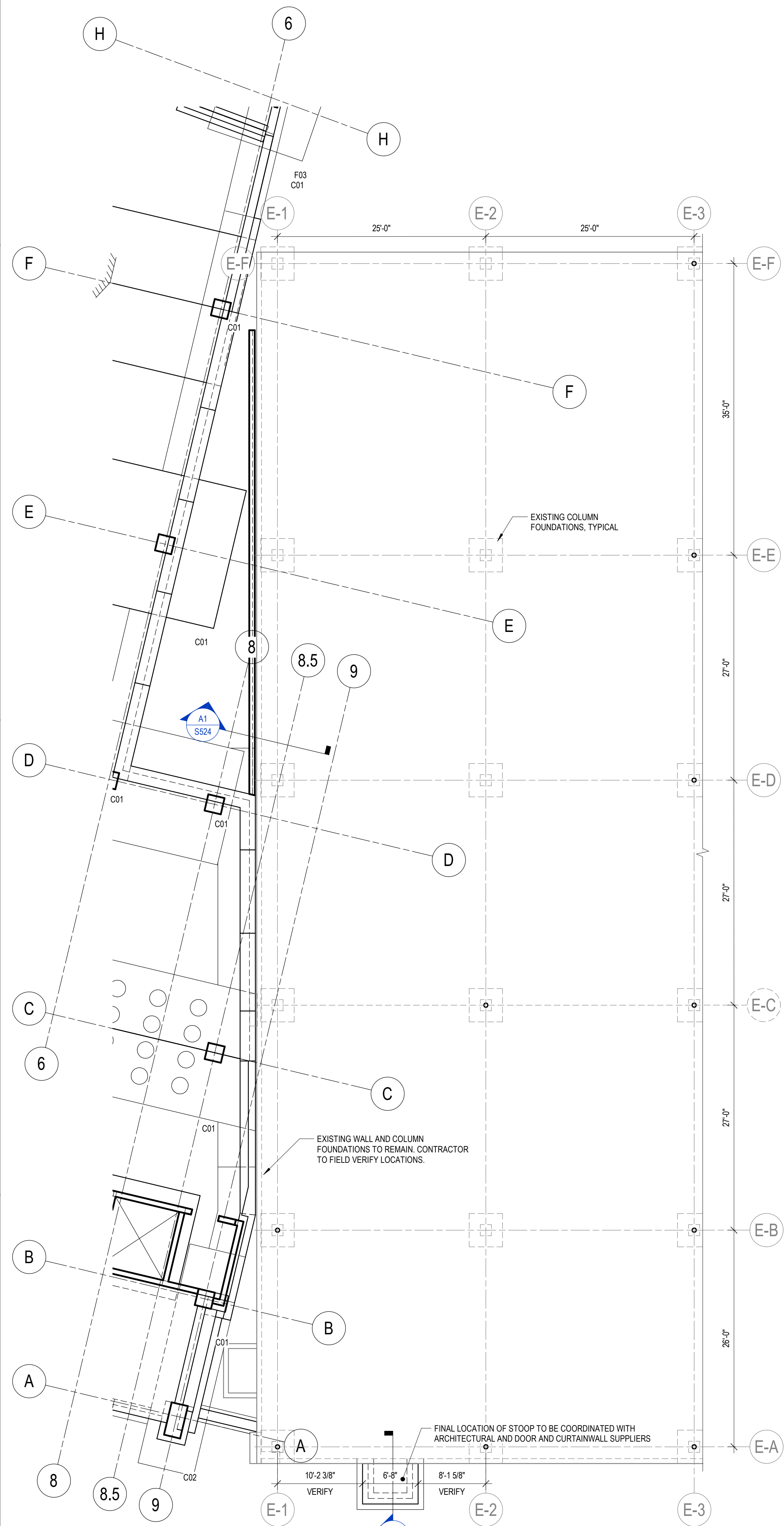


GENERAL SHEET NOTES

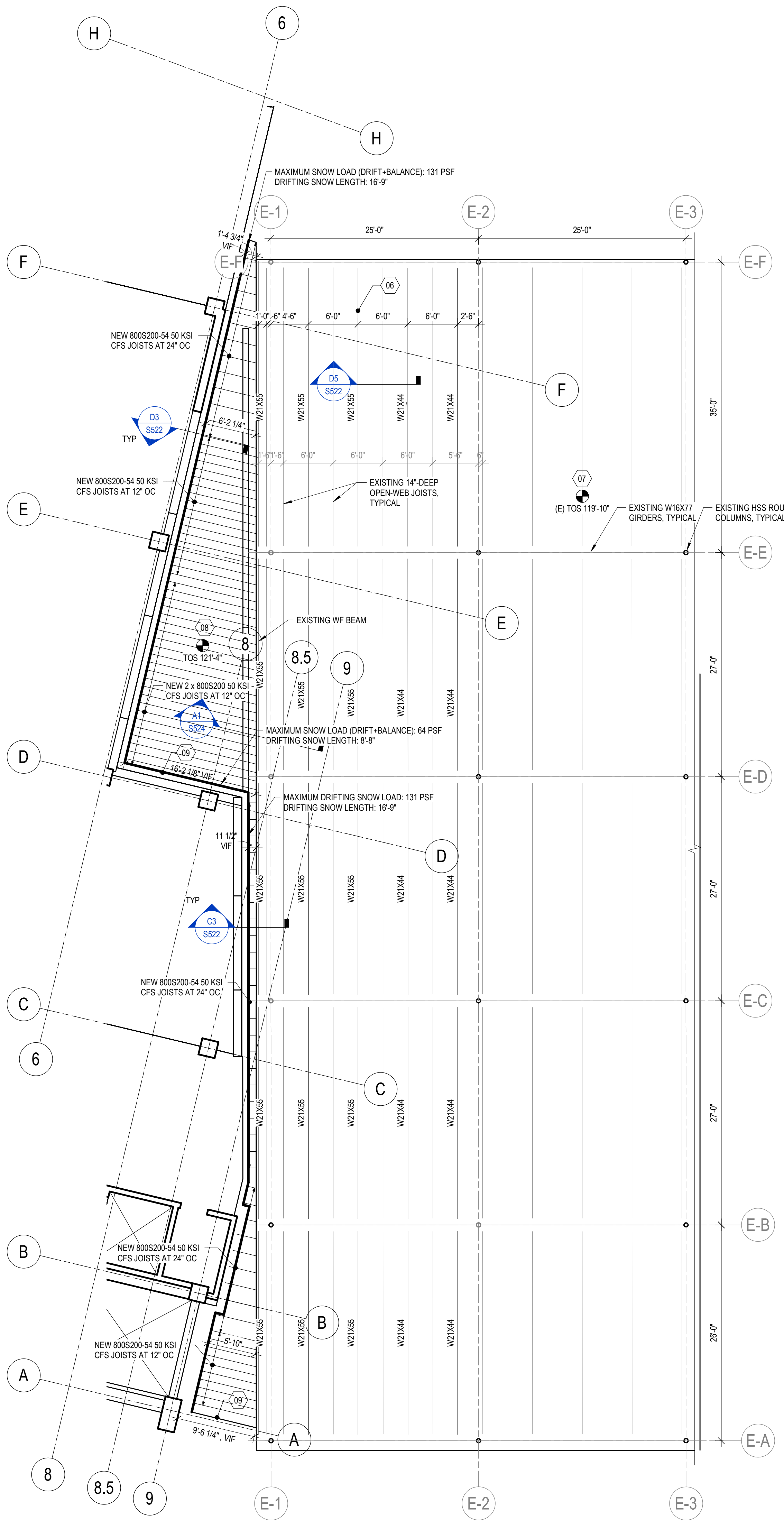
- EXISTING STRUCTURE CONSTRUCTION OF METAL ROOF WORK ON OPEN-WEB BAR JOISTS, SEE PLAN.
- TOP OF STEEL ELEVATIONS AS NOTED ON PLAN.
- SEE "SS22 - STRUCTURAL DETAILS" SHEET FOR THE FOLLOWING DETAILS:
 - TYPICAL WELDED BEAM SPICE
 - TYPICAL CONNECTION OF BEAM TO WF GIRDER
 - TYPICAL JOIST TO WF BEAM TRANSFER DETAIL
- EXISTING SUSPENDED CEILING, ELECTRICAL, MECHANICAL, AND PLUMBING COMPONENTS TO BE TEMPORARILY REMOVED AND REINSTALLED WITHIN AFFECTED ZONE OF EXISTING COMMERCIAL SPACE TO FACILITATE INSTALLATION OF NEW STRUCTURAL STEEL MEMBERS AND DETAILS.
- EXISTING BUILDING NOTE: INFORMATION PERTAINING TO THE EXISTING CONDITIONS PROVIDED IN THESE CONSTRUCTION DOCUMENTS REPRESENTS, TO THE BEST OF THE KNOWLEDGE OF THE ARCHITECT, ALL EXISTING CONDITIONS OF THE PROJECT. ANY DISCREPANCIES OF EXISTING CONDITIONS SHALL BE FIELD VERIFIED AND COORDINATED PRIOR TO THE FABRICATION OF STRUCTURAL COMPONENTS. DISCREPANCIES BETWEEN THESE DRAWINGS AND FIELD CONDITIONS SHALL BE THE RESPONSIBILITY OF THE ARCHITECT/ENGINEER FOR REVIEW. ANY WORK PERFORMED PRIOR TO DISCOVERY OF DISCREPANCIES BY THE ARCHITECT/ENGINEER IS SUBJECT TO REMOVAL AND REPLACEMENT AT NO ADDITIONAL COST TO THE CONTRACT. EXISTING STRUCTURAL COMPONENTS AND ANCHORS SHALL BE FIELD VERIFIED THAT ALL EXISTING STRUCTURAL COMPONENTS MATCH EXISTING PLAN.
- CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, ERECTION AND SEQUENCING OF ALL TEMPORARY SHORING, ALL LOCATIONS OF TEMPORARY BRACING, SHORING AND UNDERPINNING IS TO BE DETERMINED BY THE CONTRACTOR AND THE CONTRACTORS ENGINEER. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- THE CURRENT TENANTS LEASE EXPIRES ON AUGUST 31ST 2023, THEREFORE THE WORK SHALL NOT COMMENCE UNTIL THEY VACATE THE SPACE.
- IN ORDER TO TURN AROUND THE SPACE TO THE PROSPECTIVE NEW GROCERY TENANT, THE CONTRACTOR SHOULD SUPPORT WORK IN THE GROCERY STORE SPACE SHALL OCCUR SHORTLY AFTER THE END OF 2023 AND BE NOTED IN THE SCHEDULING OF THE SPACE SHOULD BE COMPLETED IN A TIMELY MANNER TO MINIMIZE AND DOWN TIME.

SHEET KEYNOTES

- 06 BEAM SPLICES AT CONTRACTOR'S OPTION, TYPICAL. SPLICED SECTIONS SHALL BE NO LESS THAN 5'-0".
- 07 EXISTING 1" ROOF DECK AND INSULATION.
- 08 NEW 1 1/2" 20GA ROOF DECK WITH SLOPING INSULATION. FASTEN WITH #12 TEK SCREWS IN 36/4 PATTERN.
- 09 PROVIDE CONTINUOUS GALVANIZED L3x3x1/8" FOR DECK BEARING. FASTEN TO CMJ OR CONCRETE WALL USING 1/2" DIA x 4" LONG HITL KWIK EZ CONCRETE SCREW ANCHORS AT 16" OC MAX.



LEVEL ONE EXISTING FOUNDATION PLAN



42 EXISTING ROOM FRAMING PLAN

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DRAWN BY: JL

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APPROVED BY: DFW

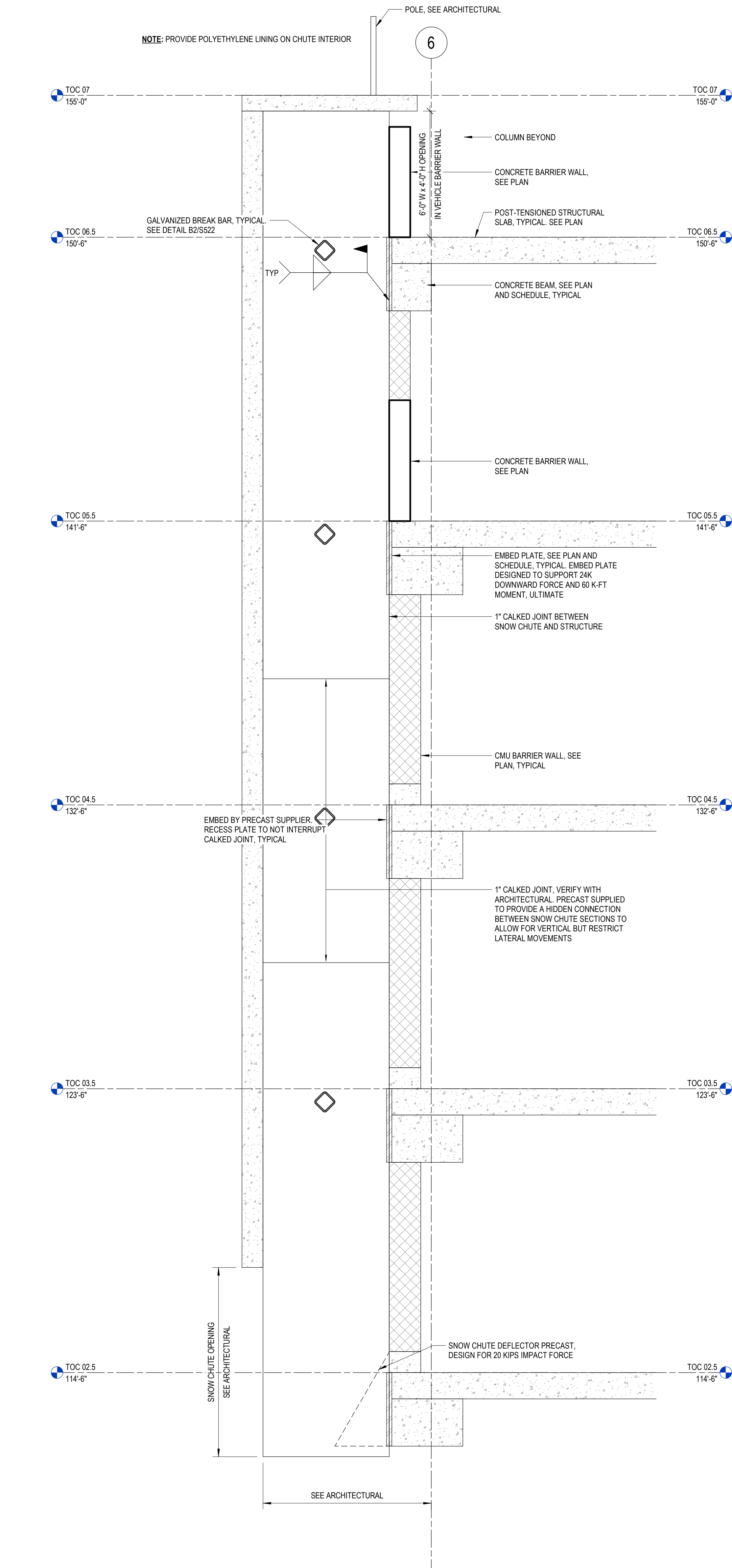
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SHEET TITLE:

BUILDING SECTIONS

SHEET NUMBER:

S201



A5 SECTION THROUGH SNOW CHUTE
1/2" = 1'-0"

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1

2

3

4

5

6

1

2

3

4

5

6

GENERAL SHEET NOTES

- TYPICAL FLOOR = 10" TWO-WAY FLAT SLAB. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE
- TOP OF CONCRETE SLAB ELEVATION AS NOTED ON PLAN
- SEE "SCHEDULES" SHEET FOR CONCRETE BEAM AND SHEARHEAD SCHEDULES.
- MAINTAIN SPECIFIED SLAB THICKNESS THROUGHOUT FLAT SLAB UNLESS NOTED OTHERWISE.
- SEE ELECTRICAL, PLUMBING, FIRE PROTECTION, HVAC AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF SLAB OPENINGS AND PIPE SLEEVES. PROVIDE SLEEVES WITH DIAMETER 1" LARGER THAN CONDUIT. ALL SLEEVES TO BE SCHEDULE 40 STEEL PIPE.
- NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.

SHEET KEYNOTES

- 10 (2) #4 x 5'-0" LONG LOCATED 1" BELOW TOP OF SLAB. PROVIDE AT ALL SLAB RE-ENTRANT CORNERS. ALL LOCATIONS NOT SHOWN ON PLAN.
- 15 COLD-FORM STEEL JOISTS REFER TO A3/S108.

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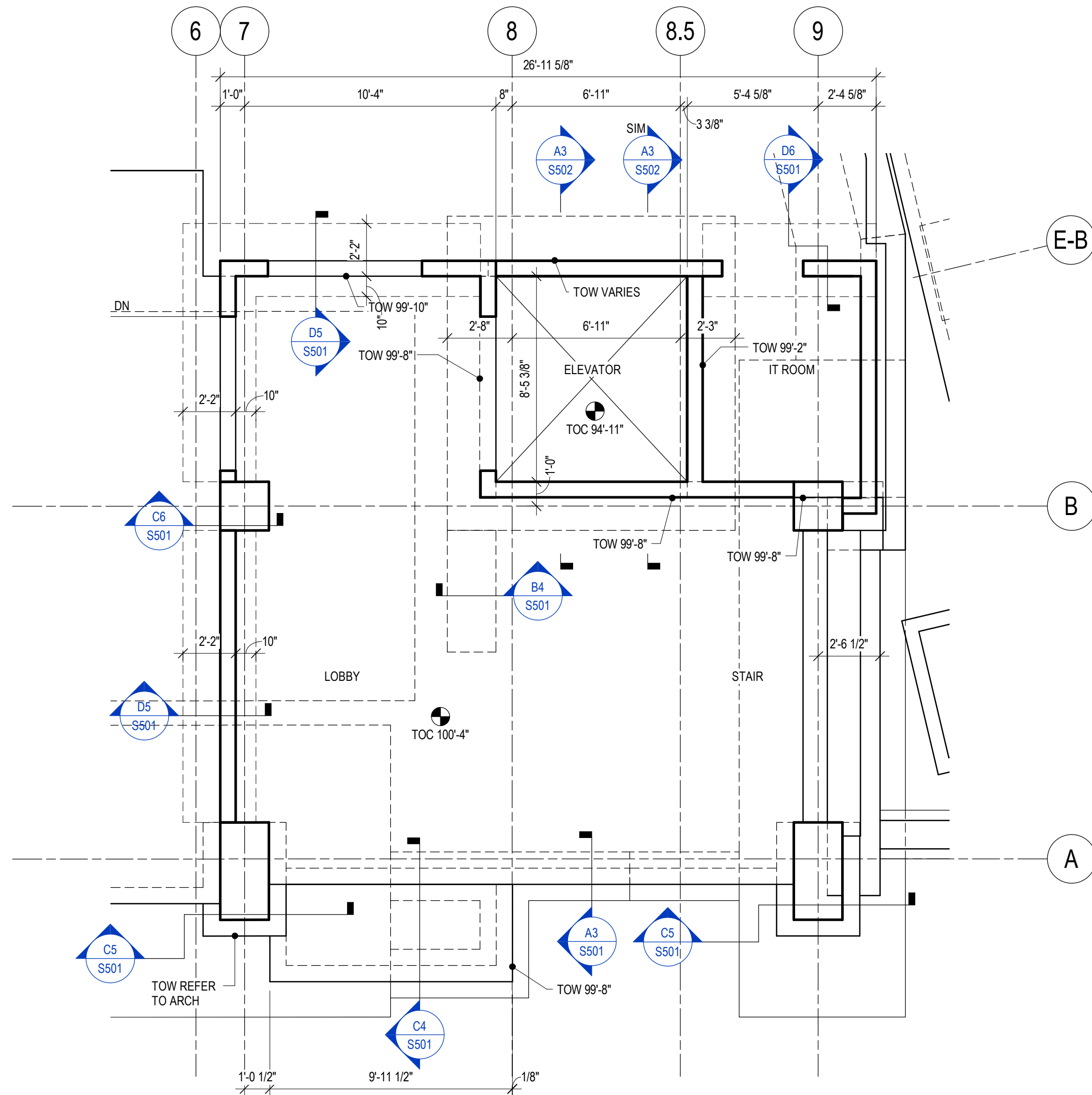
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SHEET TITLE:

ENLARGED FLOOR PLANS

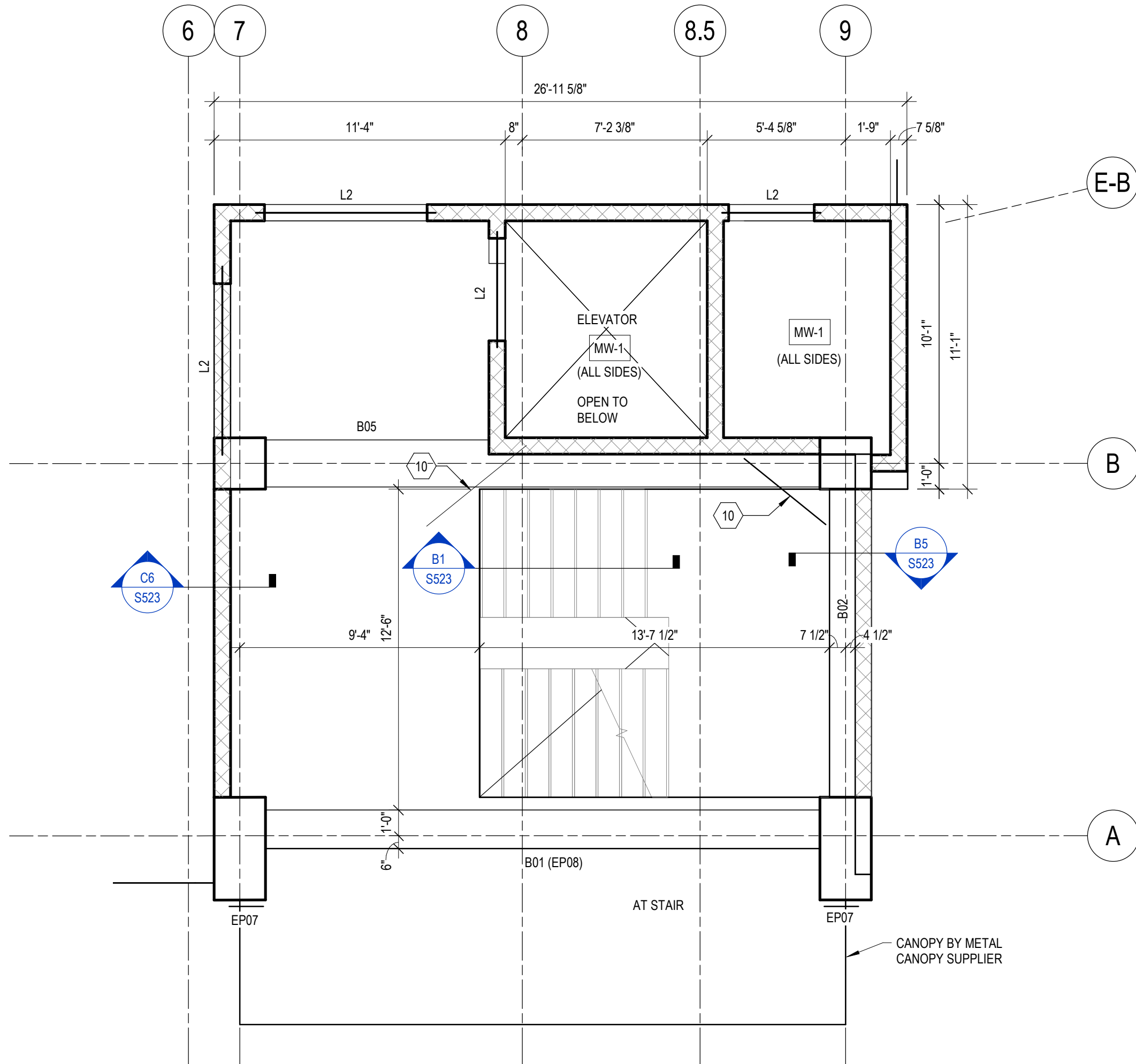
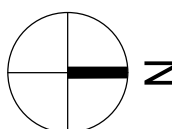
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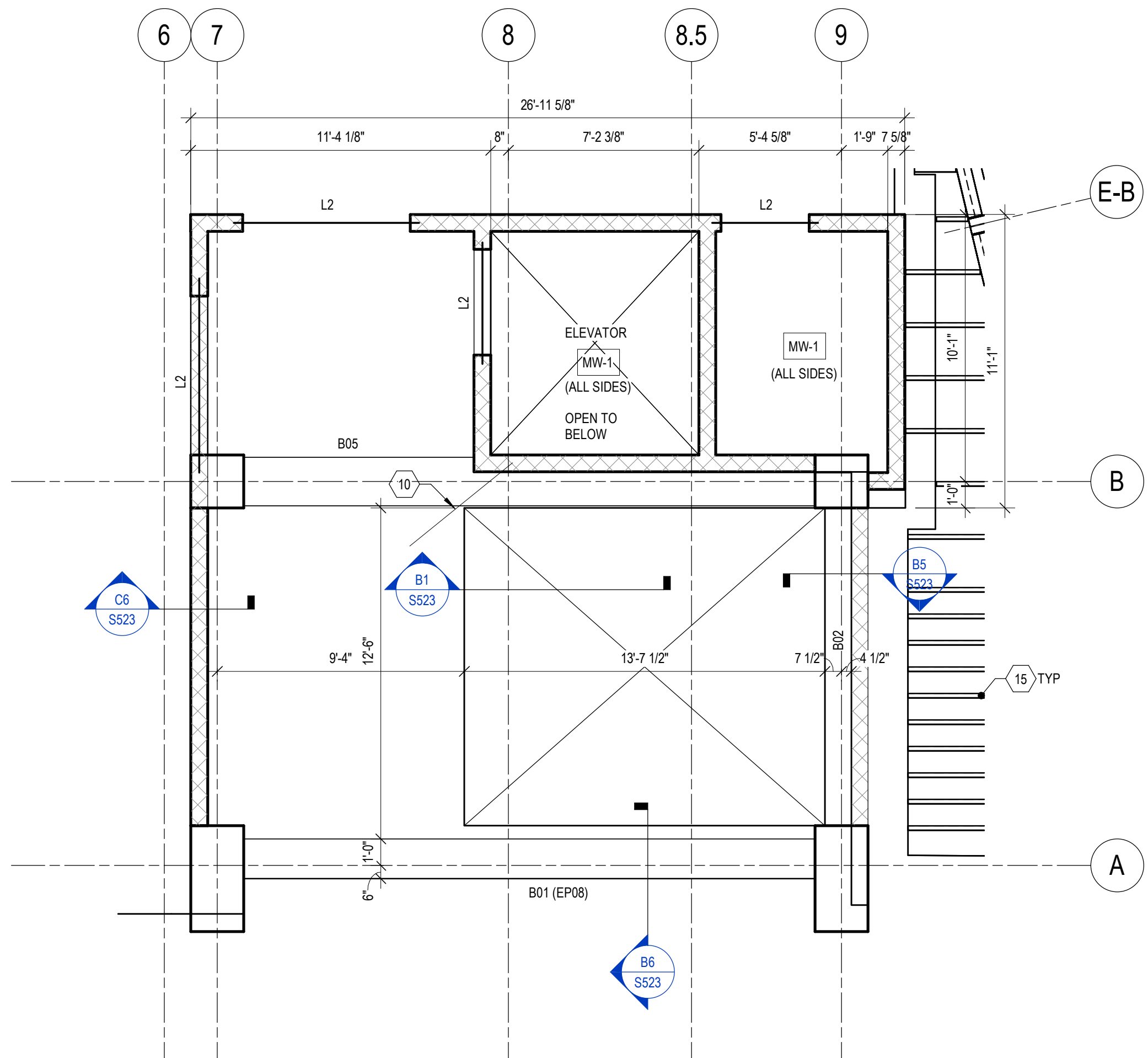
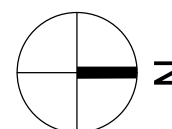
C1 NORTH STAIR AND ELEVATOR LOBBY - LEVEL ONE

1/4" = 1'-0"



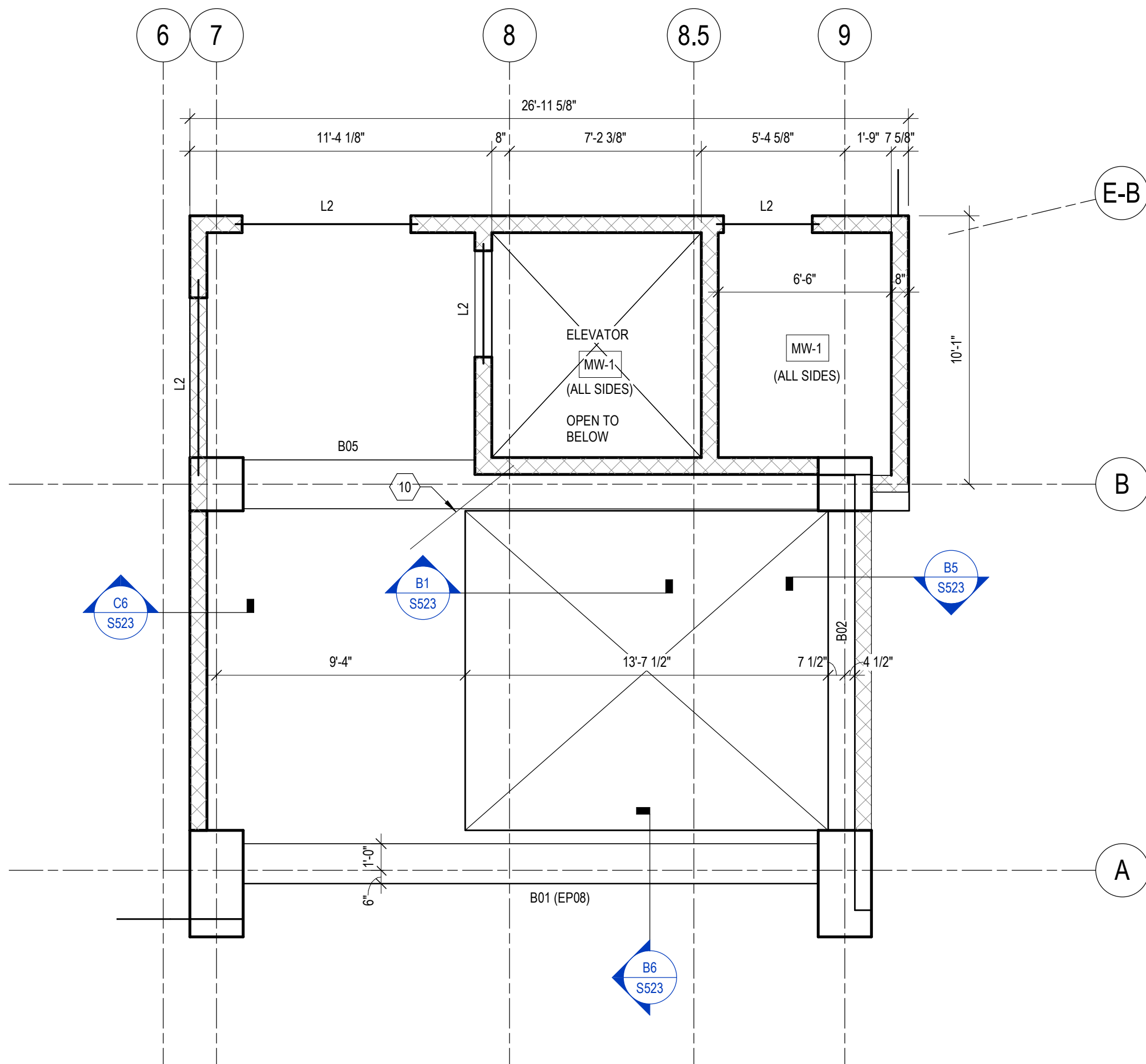
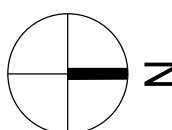
C3 NORTH STAIR AND ELEVATOR LOBBY - LEVEL TWO

1/4" = 1'-0"



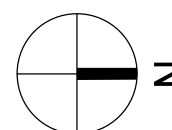
A1 NORTH STAIR AND ELEVATOR LOBBY - LEVEL THREE

1/4" = 1'-0"

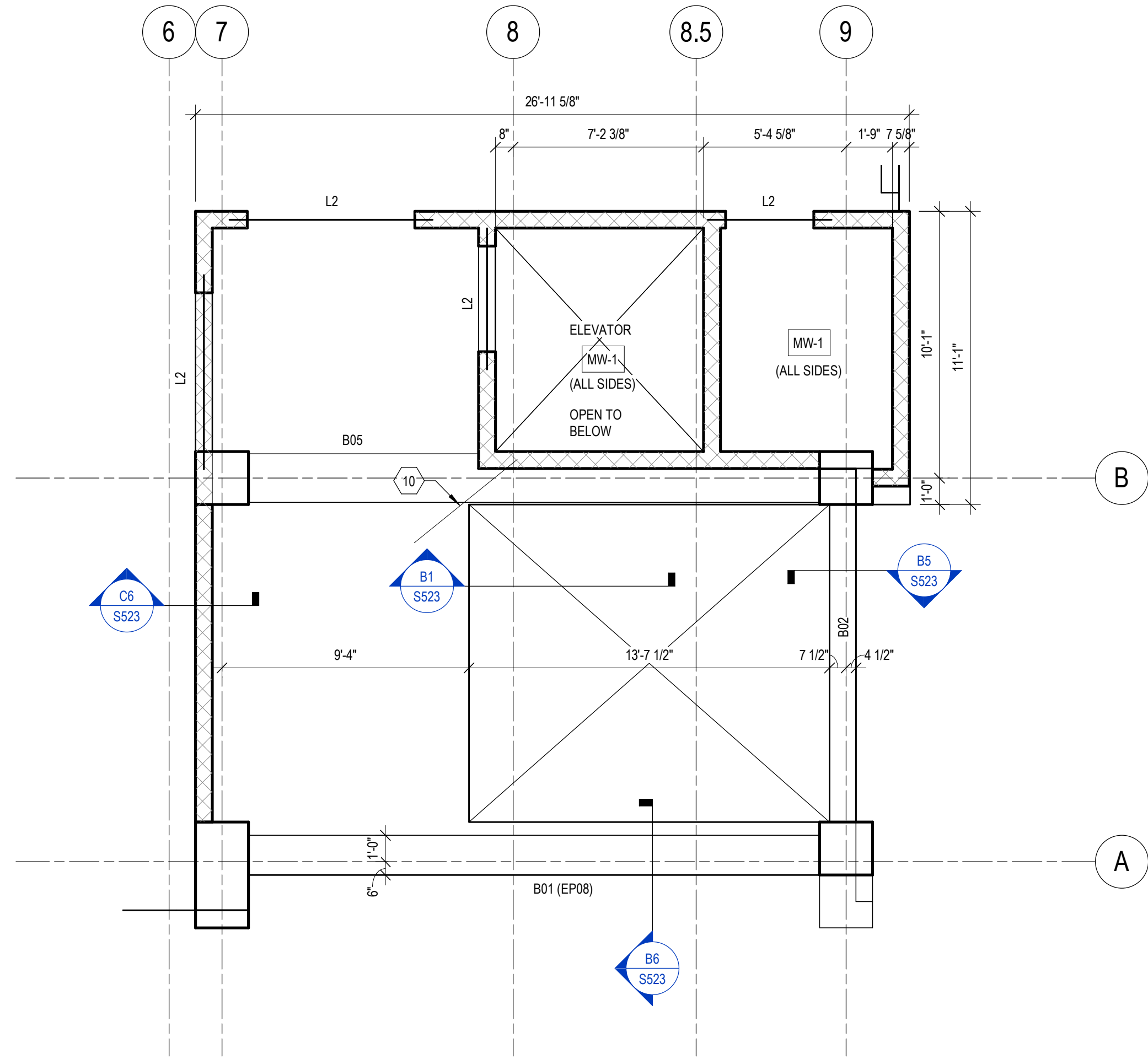


A3 NORTH STAIR AND ELEVATOR LOBBY - LEVEL FOUR

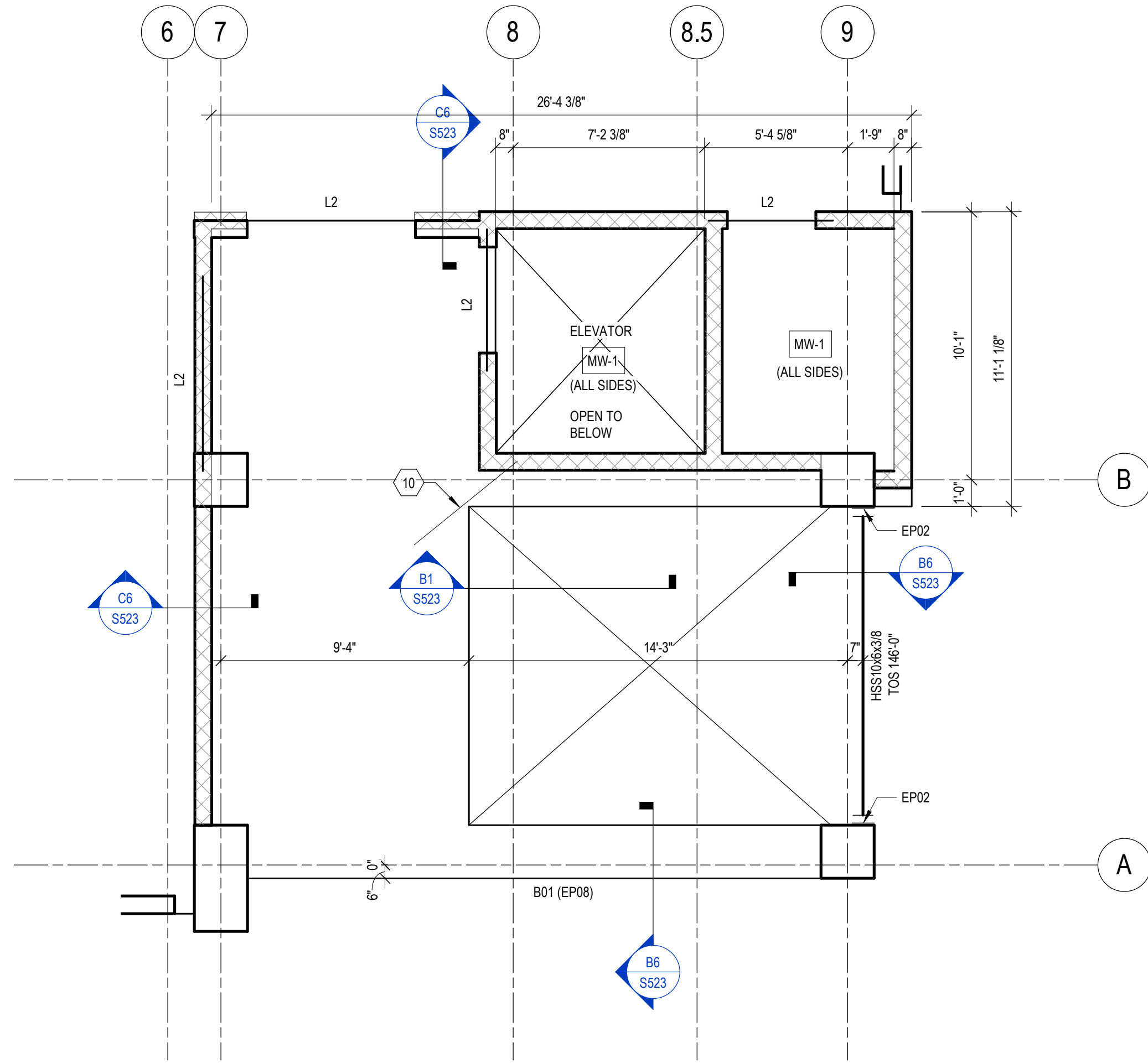
1/4" = 1'-0"



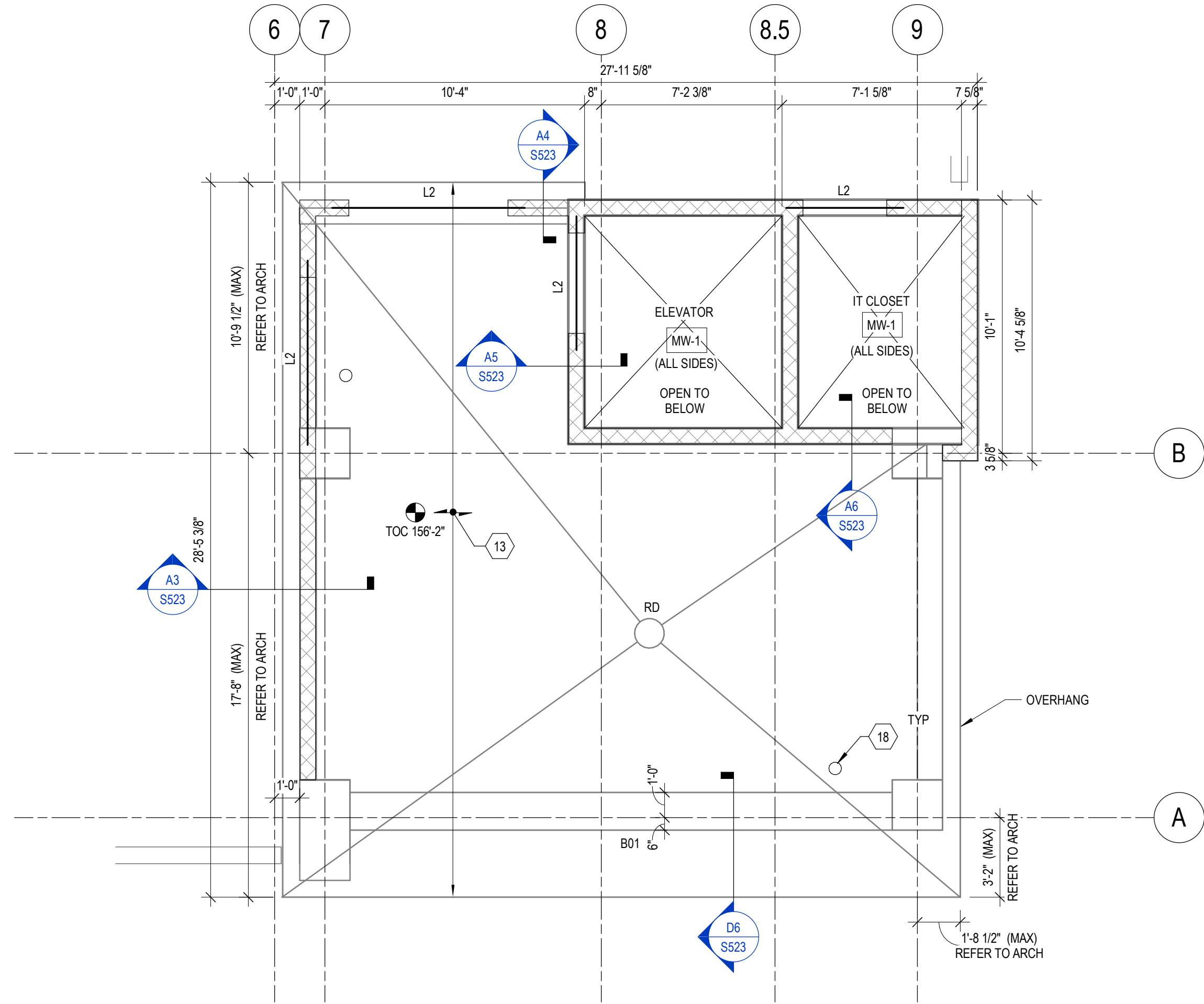
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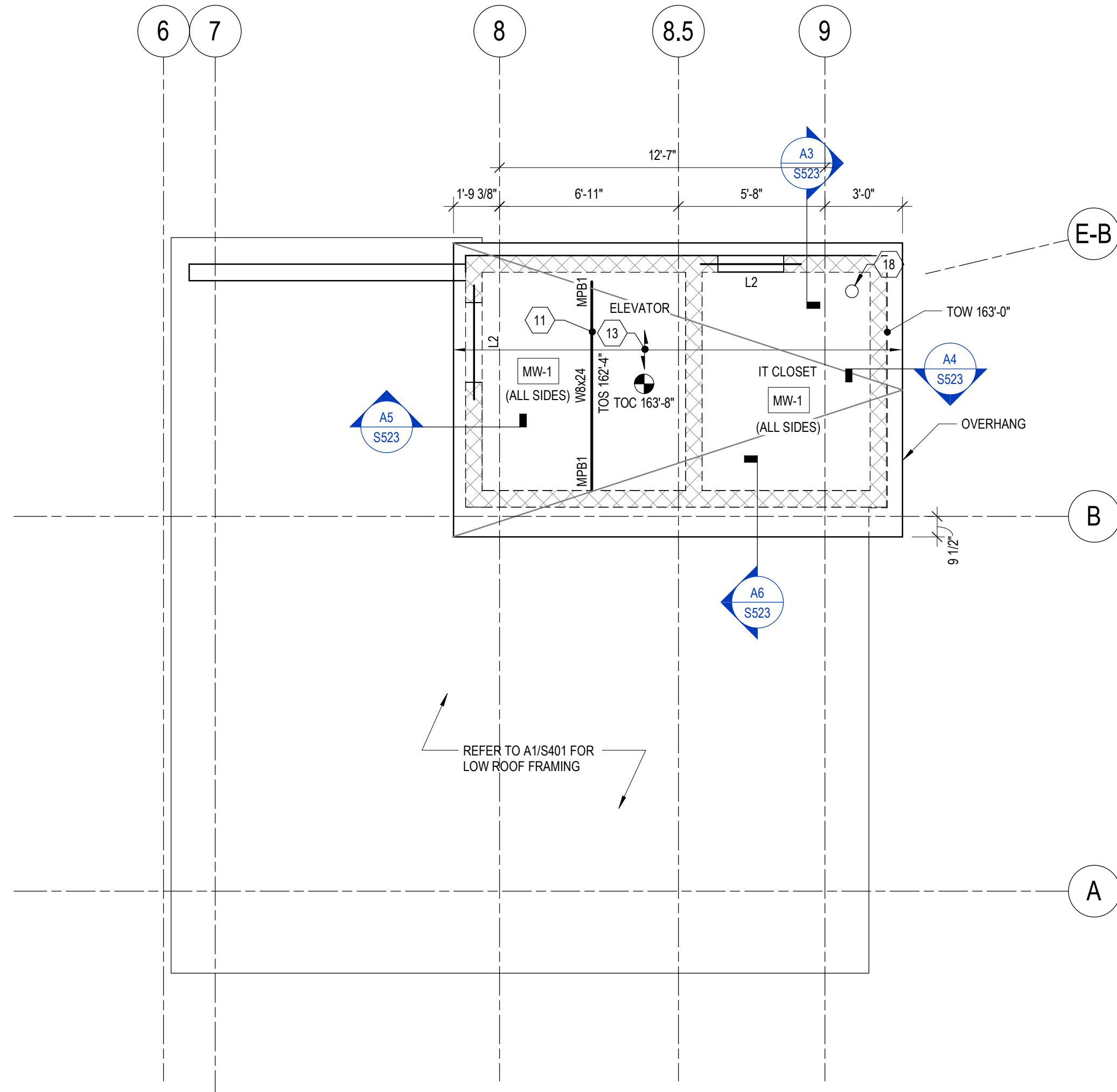
C1 NORTH STAIR AND ELEVATOR LOBBY - LEVEL FIVE
1/4" = 1'-0"



C3 NORTH STAIR AND ELEVATOR LOBBY - LEVEL SIX
1/4" = 1'-0"



A1 NORTH STAIR AND ELEVATOR LOBBY - ROOF
1/4" = 1'-0"



A3 NORTH STAIR AND ELEVATOR LOBBY - HIGH ROOF
1/4" = 1'-0"

GENERAL SHEET NOTES

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- NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.

SHEET KEYNOTES

- (2) #4 x 5'-0" LONG LOCATED 1" BELOW TOP OF SLAB. PROVIDE AT ALL SLAB RE-ENTRANT CORNERS. ALL LOCATIONS NOT SHOWN ON PLAN.
- ELEVATOR SAFETY/HOIST BEAM. VERIFY LOCATION AND ORIENTATION WITH ELEVATOR SUPPLIER.
- 8" THICK HOLLOW CORE PRECAST PLANK. HOLLOW CORE PRECAST PLANK SUPERIMPOSED DEAD LOAD OF 15 PSF. SUPERIMPOSED ROOF LIVE LOAD OF 20 PSF.
- PREFABRICATED ROOF ANCHOR/DIAVT, COORDINATE WITH OWNER'S VENDOR AND PRECAST SUPPLIER.

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APPROVED BY: DFW

SCALE: AS NOTED

SHEET TITLE:

ENLARGED FLOOR PLANS

SHEET NUMBER:

S401

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SHEET TITLE:

ENLARGED FLOOR PLANS

SHEET NUMBER:

S402

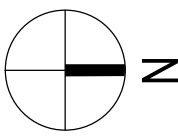
GENERAL SHEET NOTES

- TYPICAL FLOOR \approx 10' TWO-WAY FLAT SLAB. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE
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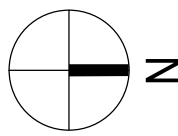
C1 SOUTH STAIR - LEVEL ONE

1/4" = 1'-0"



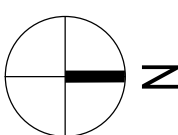
C3 SOUTH STAIR- LEVEL TWO

1/4" = 1'-0"



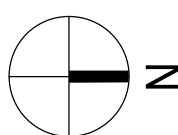
A1 SOUTH STAIR - LEVEL THREE

1/4" = 1'-0"

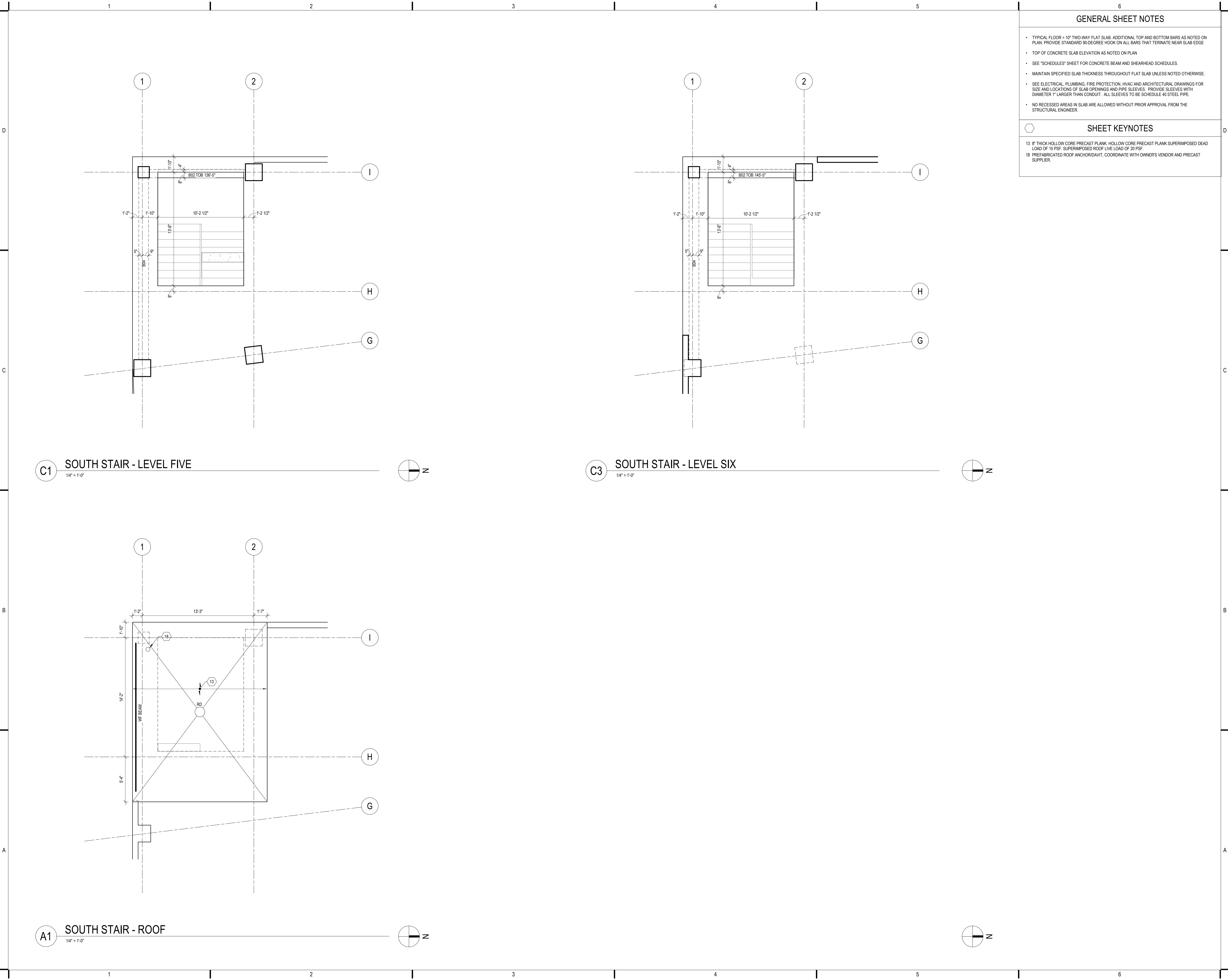


A3 SOUTH STAIR - LEVEL FOUR

1/4" = 1'-0"



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GENERAL SHEET NOTES	
<ul style="list-style-type: none">• TYPICAL FLOOR = 10" TWO-WAY FLAT SLAB. ADDITIONAL TOP AND BOTTOM BARS AS NOTED ON PLAN. PROVIDE STANDARD 90-DEGREE HOOK ON ALL BARS THAT TERMINATE NEAR SLAB EDGE• TOP OF CONCRETE SLAB ELEVATION AS NOTED ON PLAN• SEE "SCHEDULES" SHEET FOR CONCRETE BEAM AND SHEARHEAD SCHEDULES.• MAINTAIN SPECIFIED SLAB THICKNESS THROUGHOUT FLAT SLAB UNLESS NOTED OTHERWISE.• SEE ELECTRICAL, PLUMBING, FIRE PROTECTION, HVAC AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF SLAB OPENINGS AND PIPE SLEEVES. PROVIDE SLEEVES WITH DIAMETER 1" LARGER THAN CONDUIT. ALL SLEEVES TO BE SCHEDULE 40 STEEL PIPE.• NO RECESSED AREAS IN SLAB ARE ALLOWED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.	
SHEET KEYNOTES	
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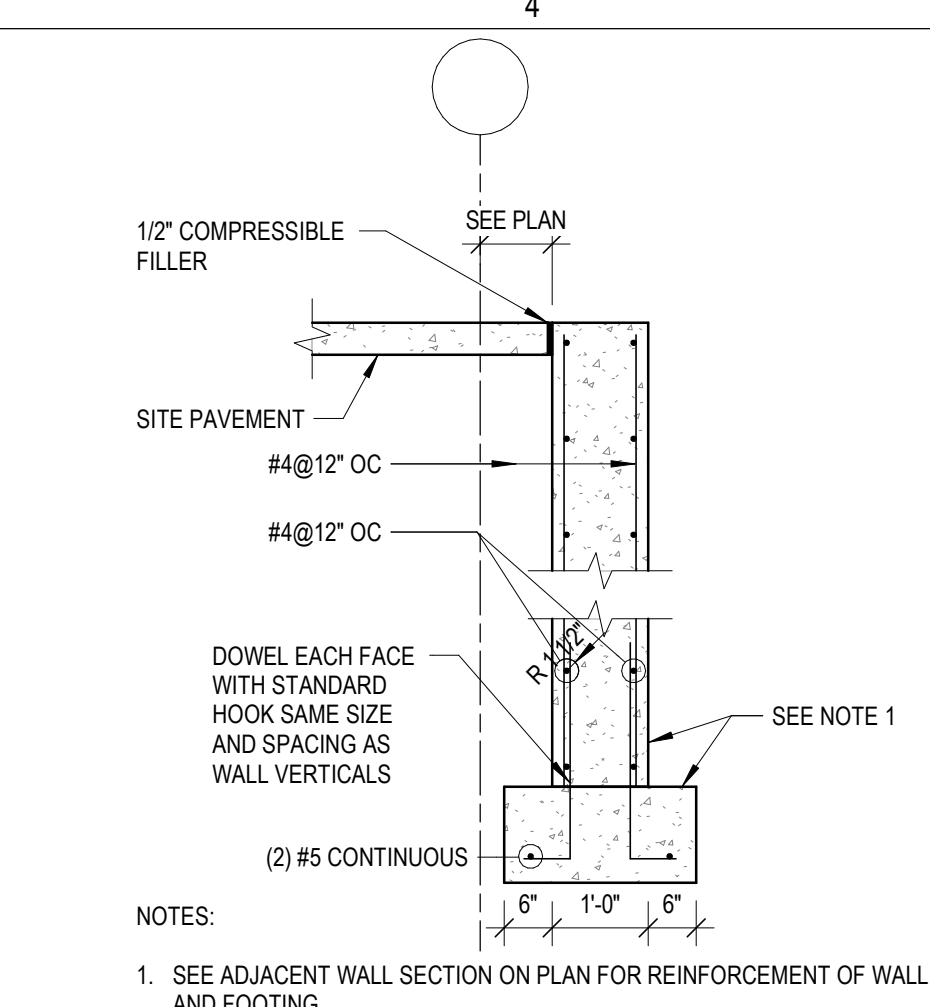
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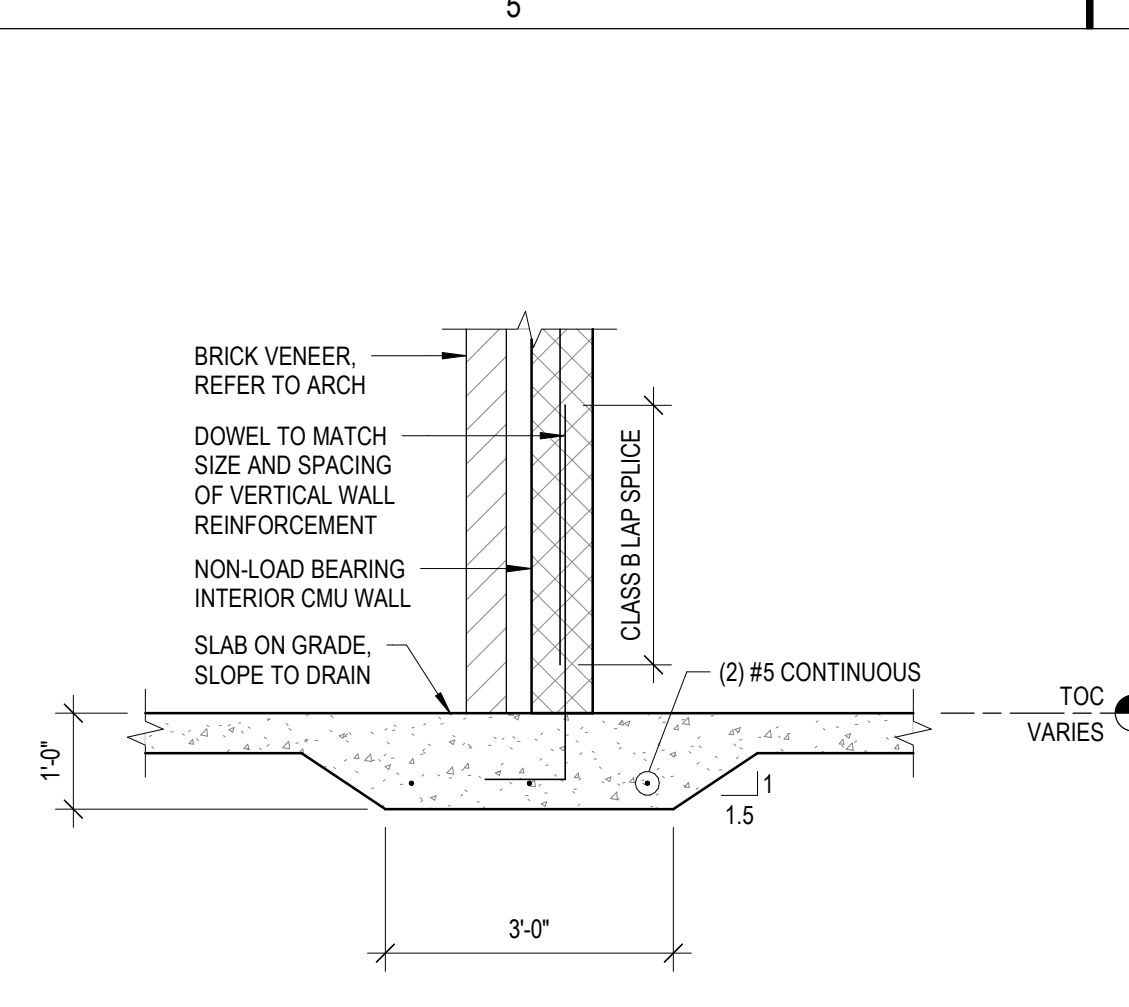
ENLARGED FLOOR PLANS

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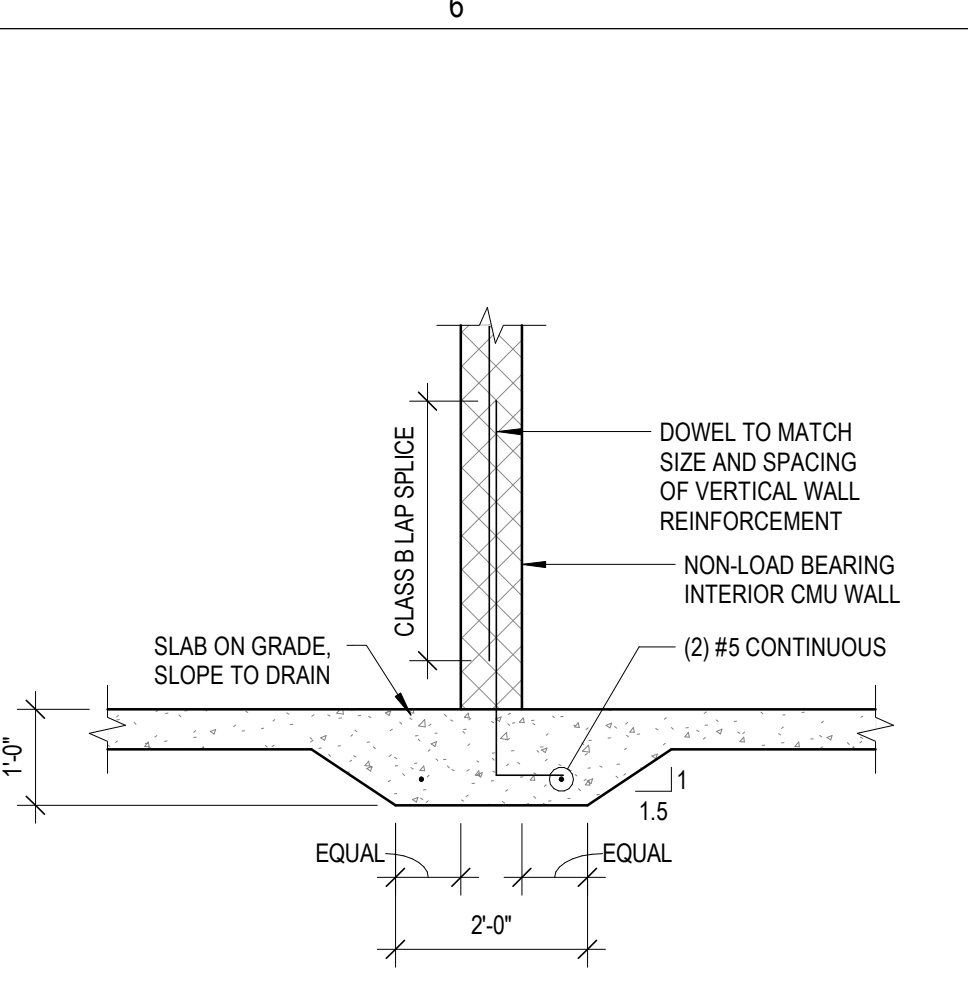
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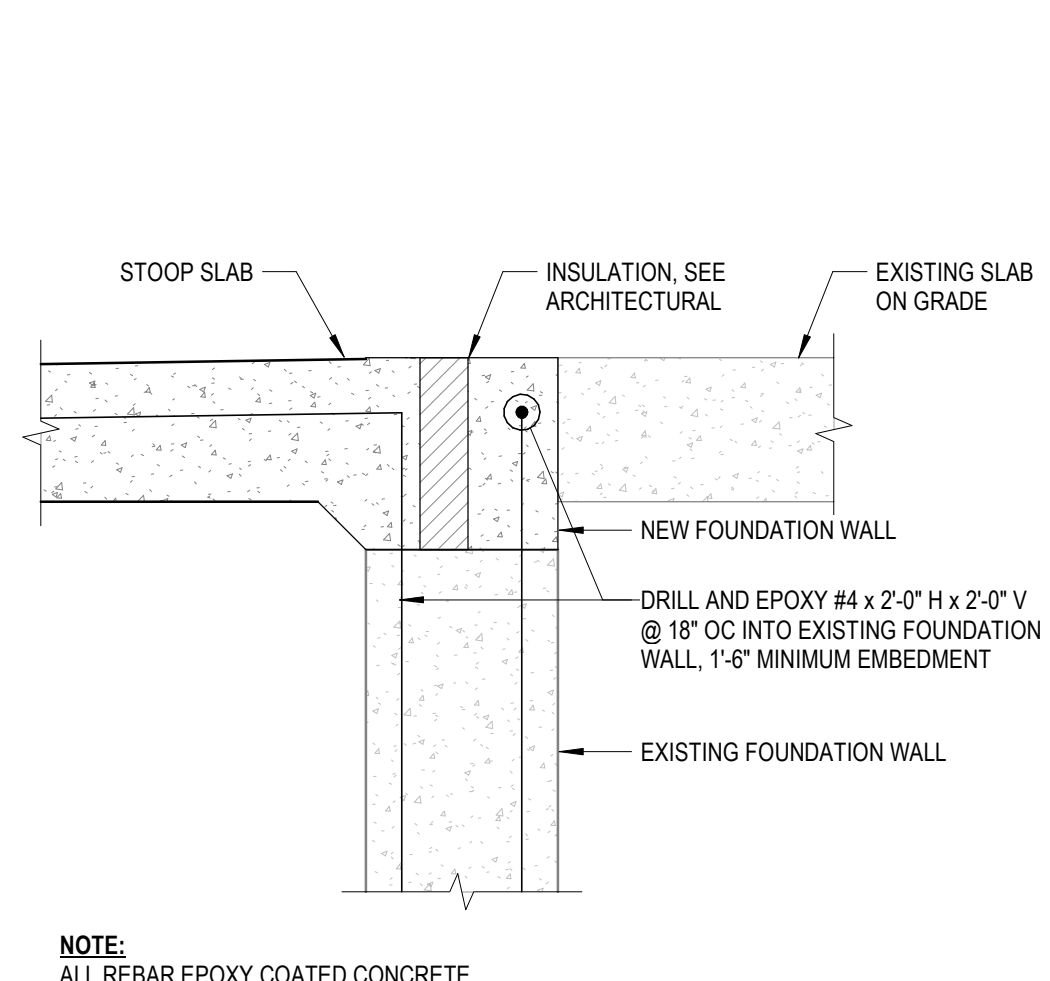
D4 DETAIL
1/2" = 1'-0"



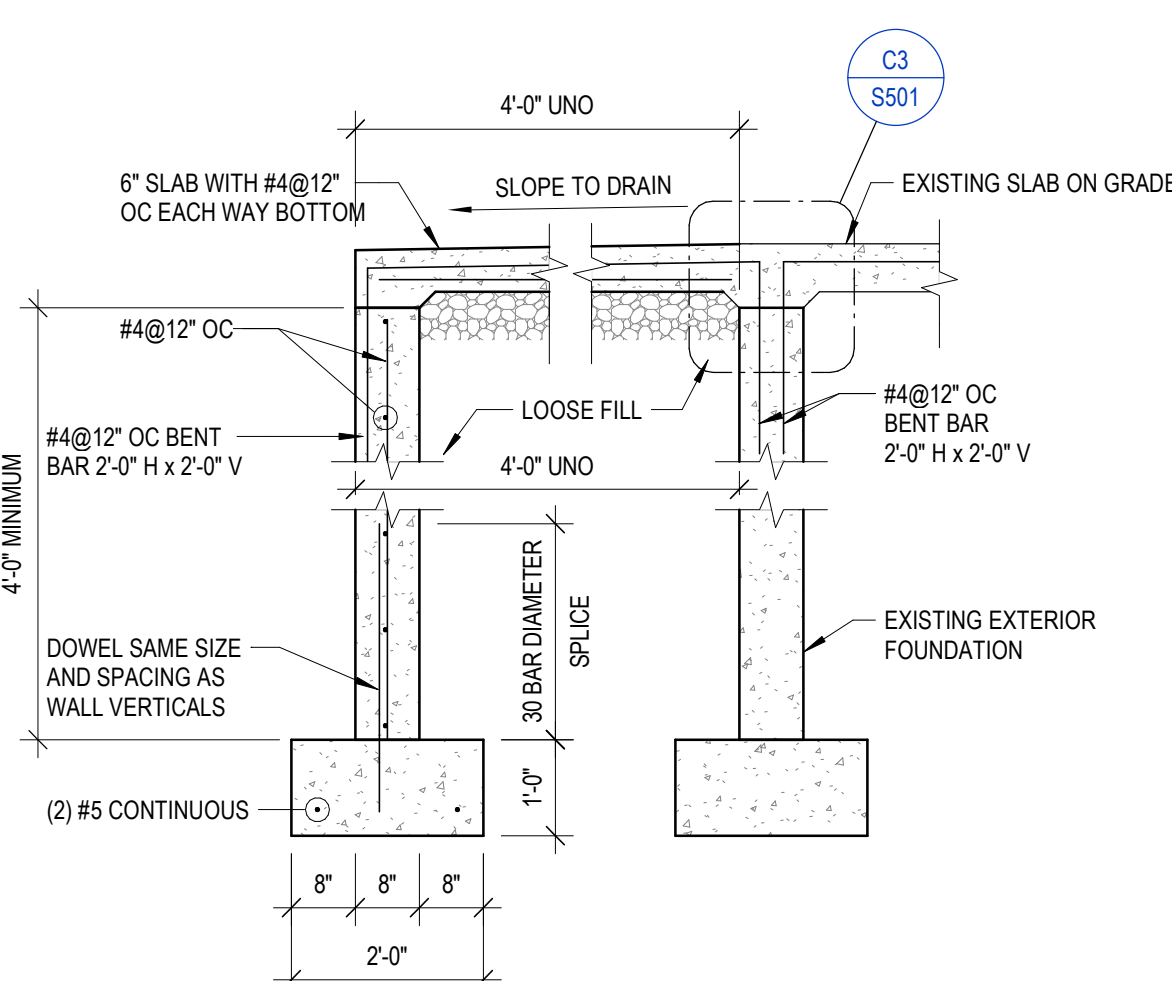
D5 CMU WALL ON SLAB ON GRADE
1/2" = 1'-0"



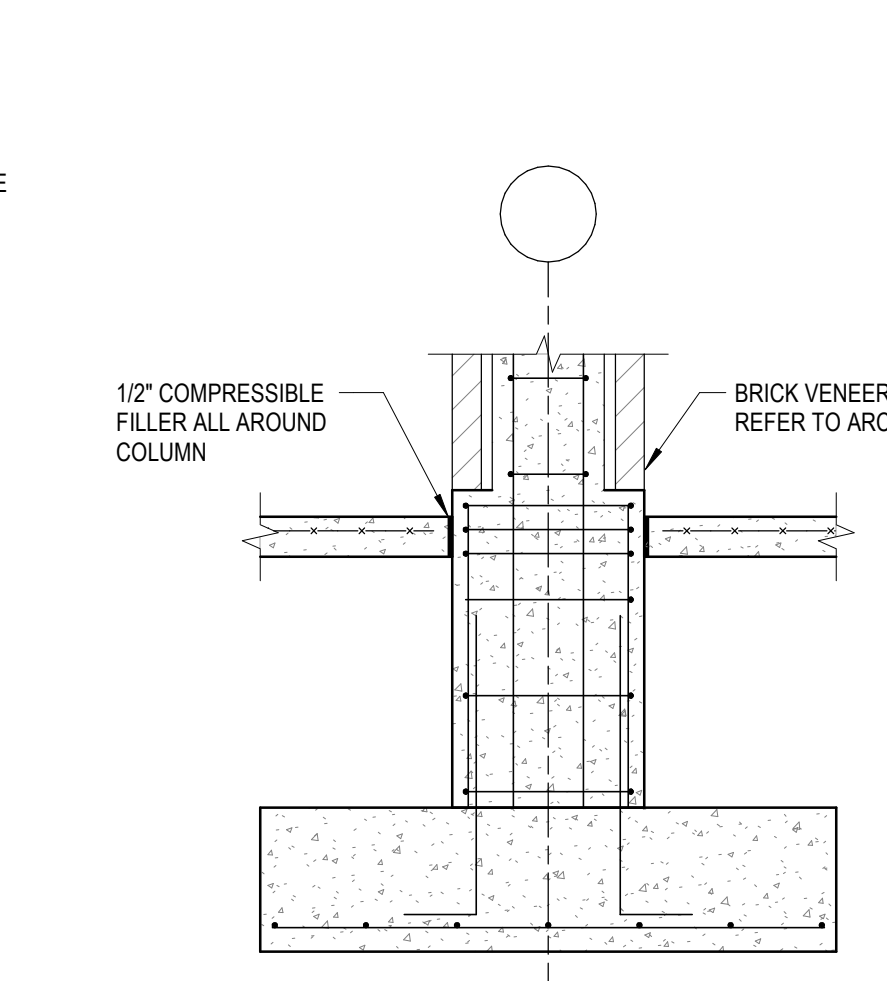
D6 CMU WALL ON SLAB ON GRADE
1/2" = 1'-0"



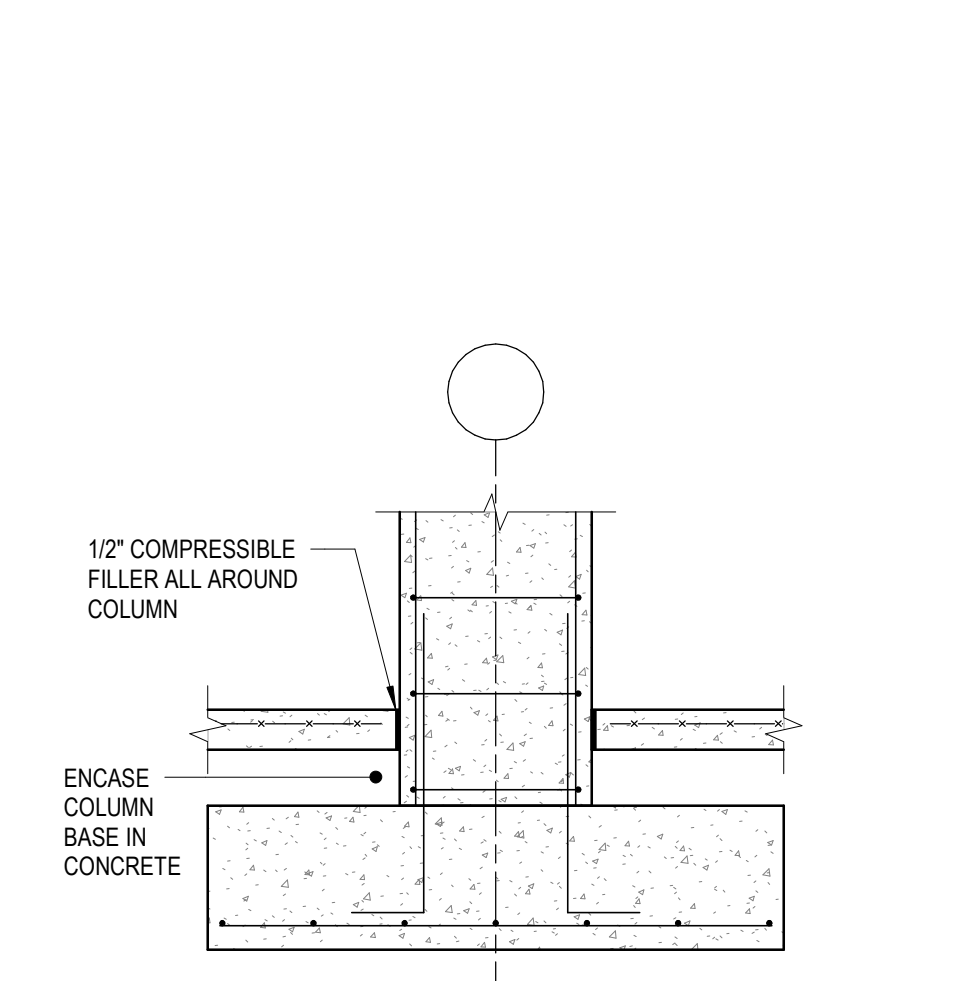
C3 THRESHOLD DETAIL AT NEW STOOP
1 1/2" = 1'-0"



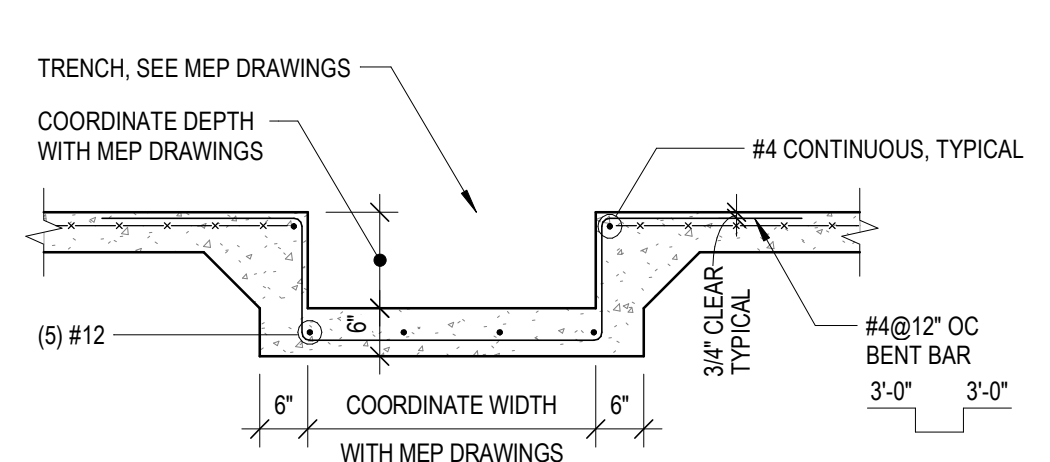
C4 TYPICAL STOOP DETAIL
1/2" = 1'-0"



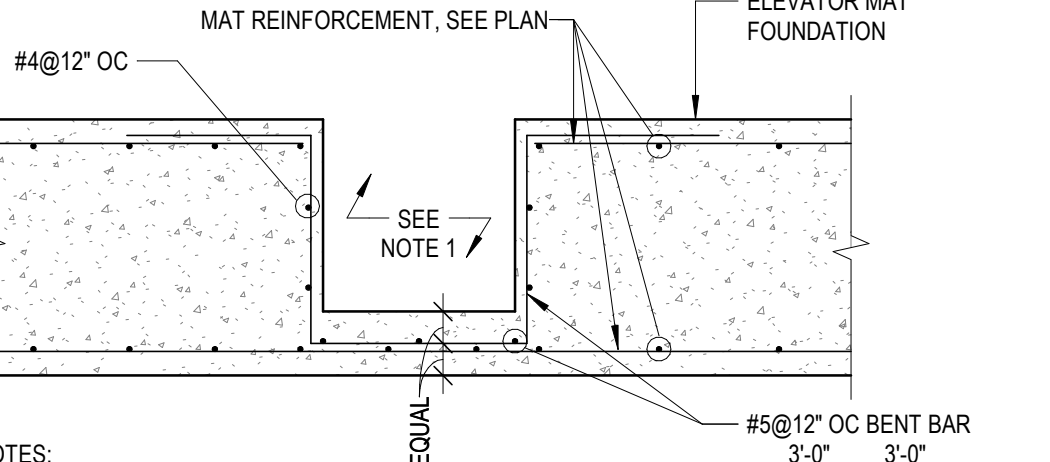
C5 DETAIL
1/2" = 1'-0"



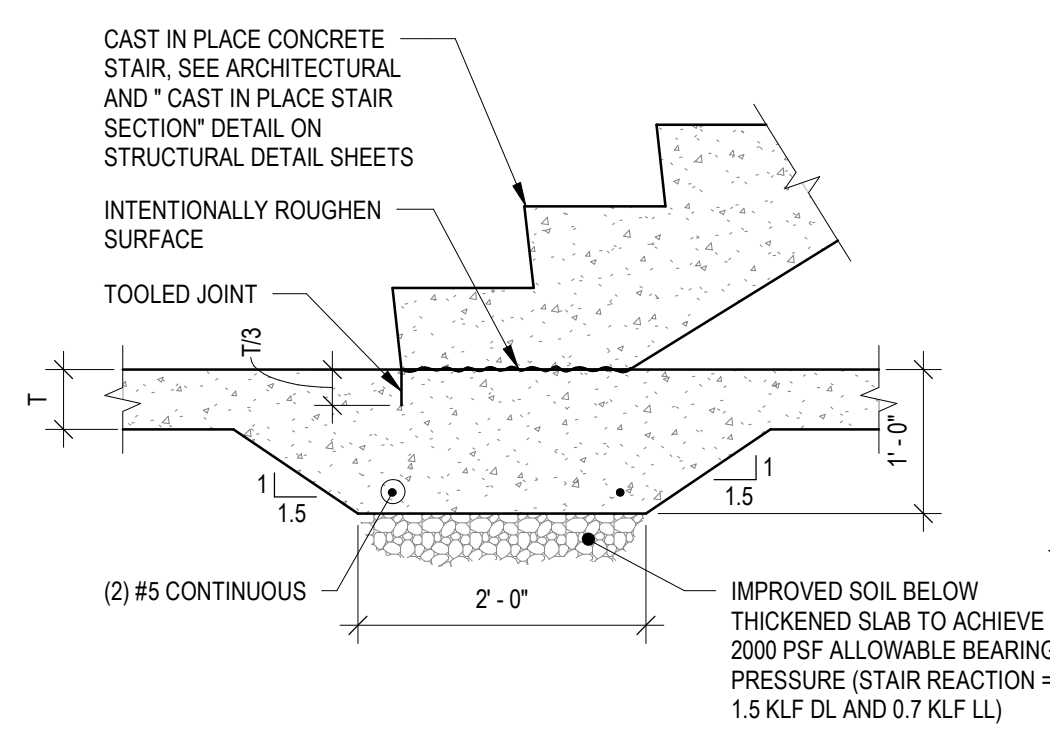
C6 DETAIL
1/2" = 1'-0"



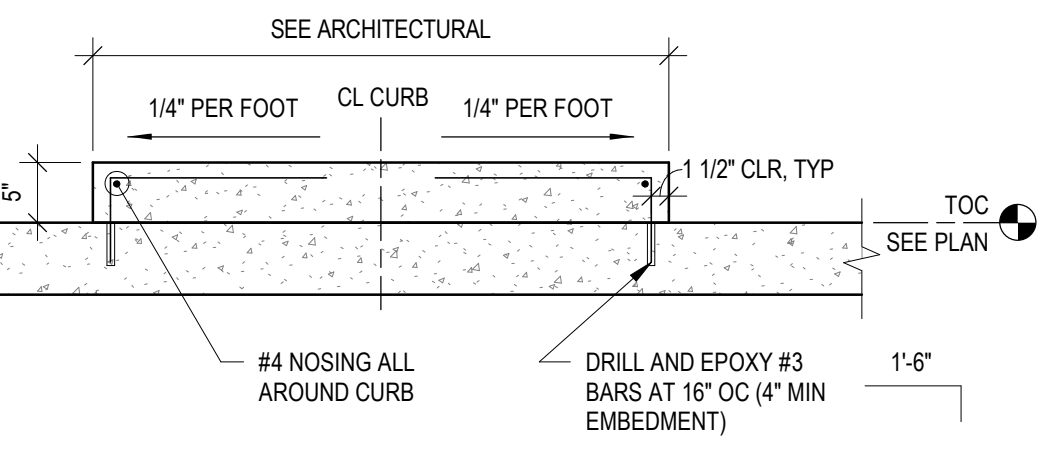
B2 DETAIL
1/2" = 1'-0"



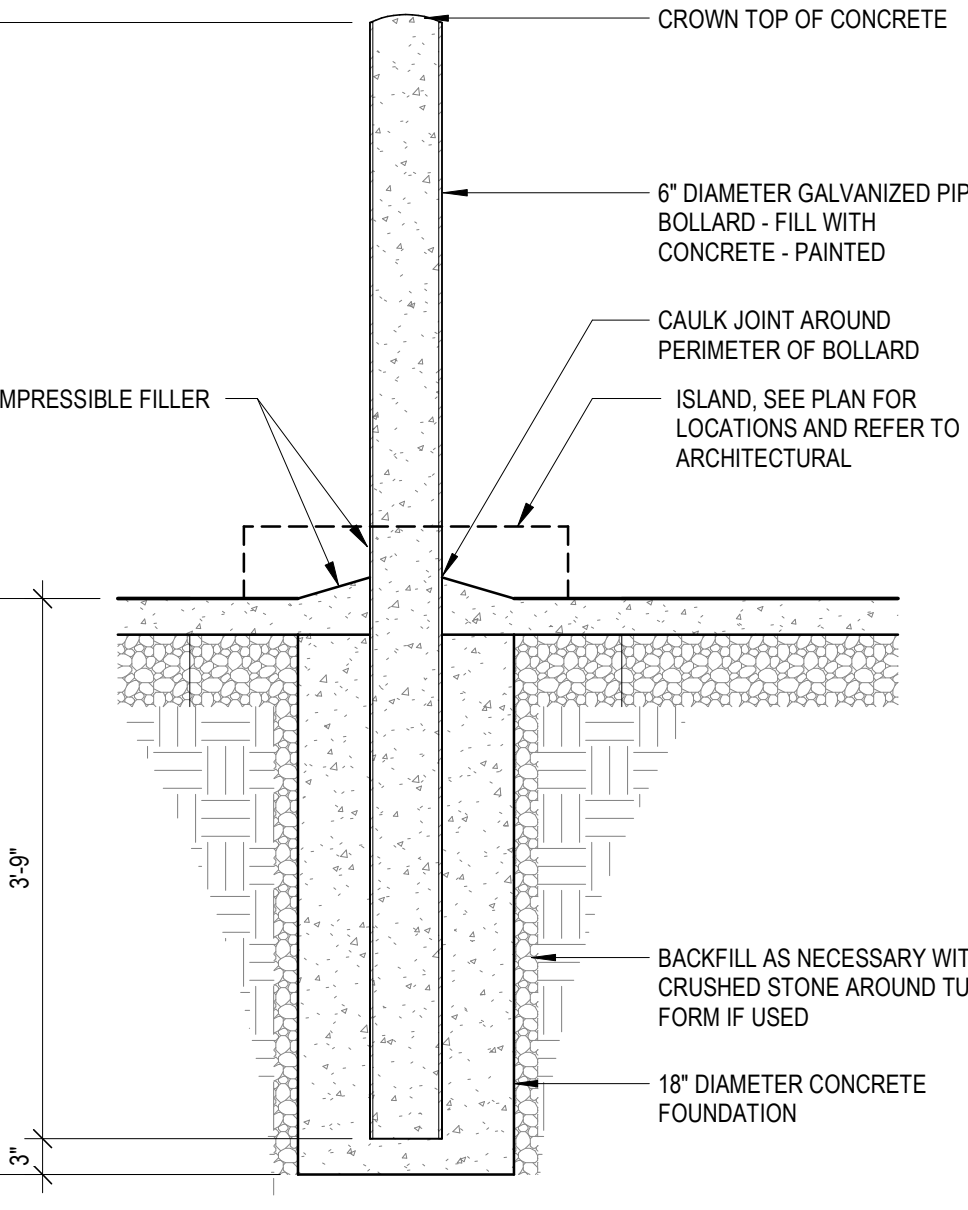
B3 DETAIL
1/2" = 1'-0"



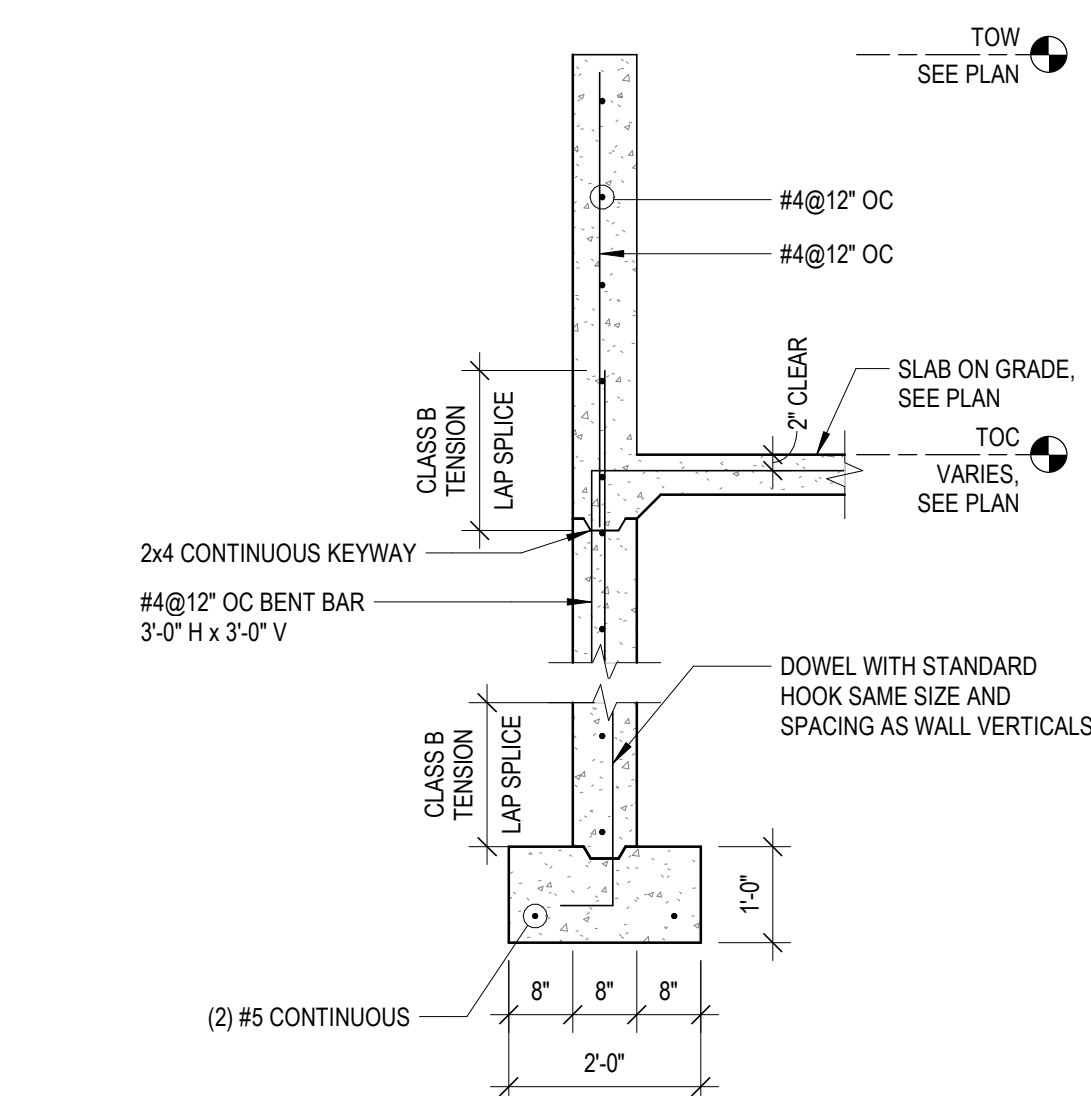
B4 THICKENED SLAB AT STAIR
3/4\" = 1'-0"



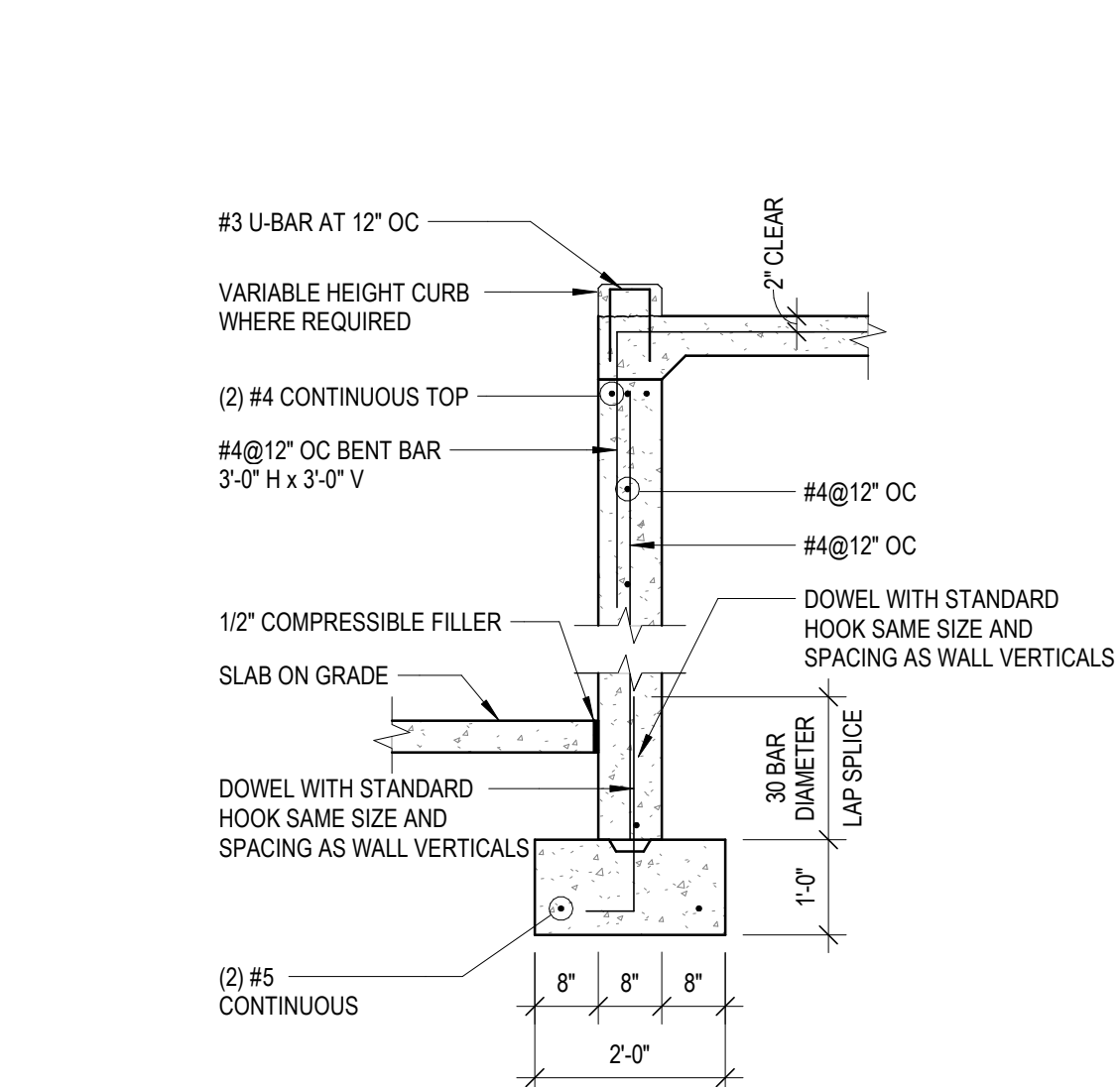
B5 TYPICAL CURB DETAIL
3/4\" = 1'-0"



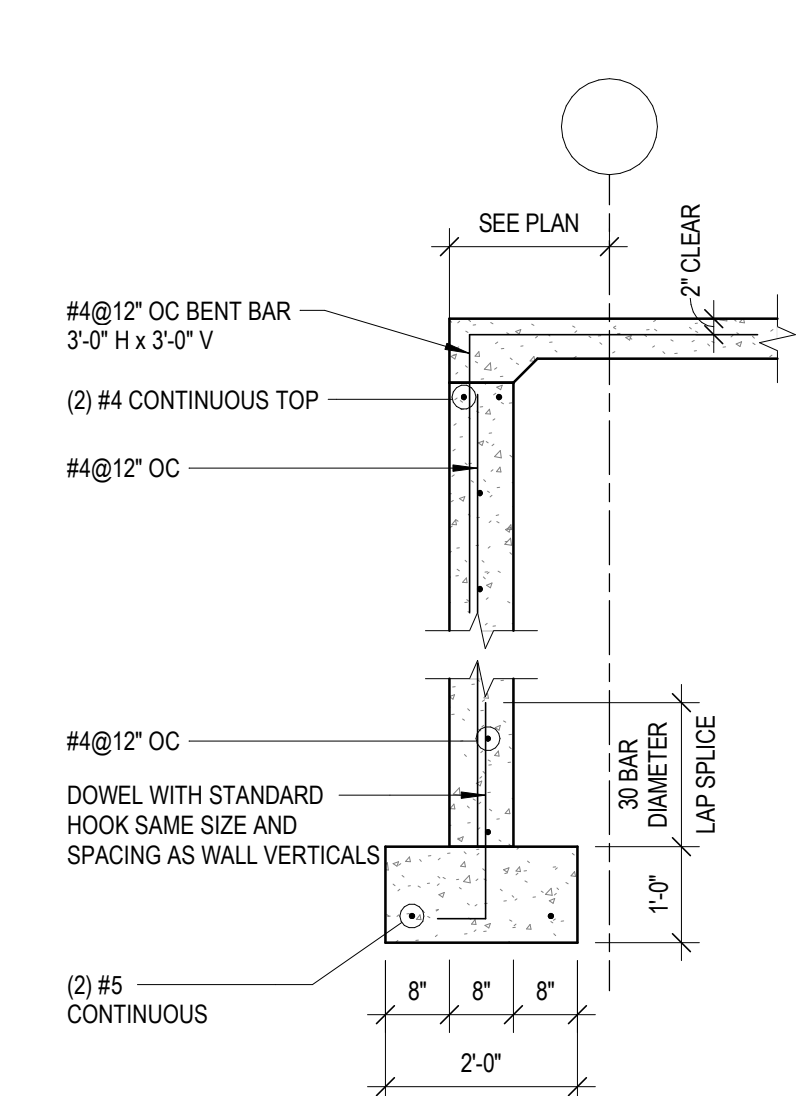
B6 BOLLARD DETAIL AT GRADE
3/4\" = 1'-0"



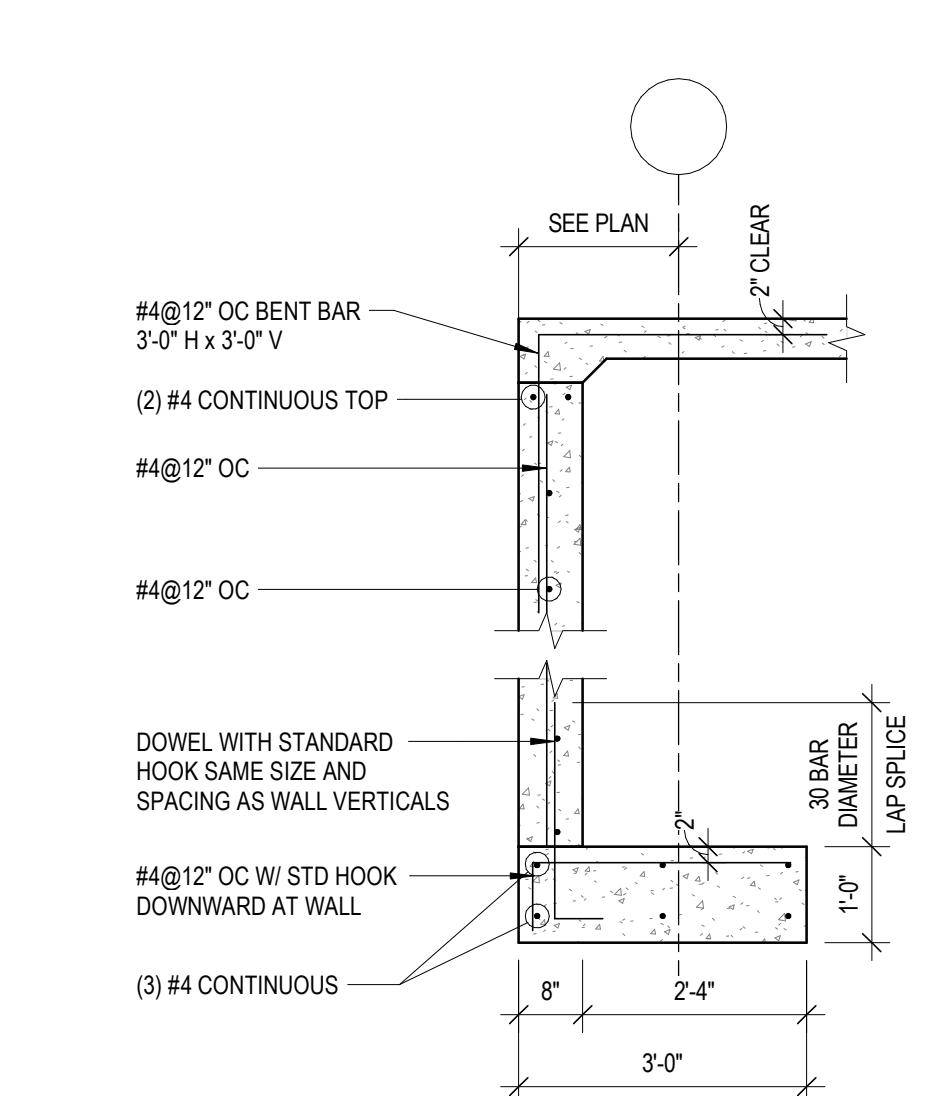
A1 DETAIL
1/2" = 1'-0"



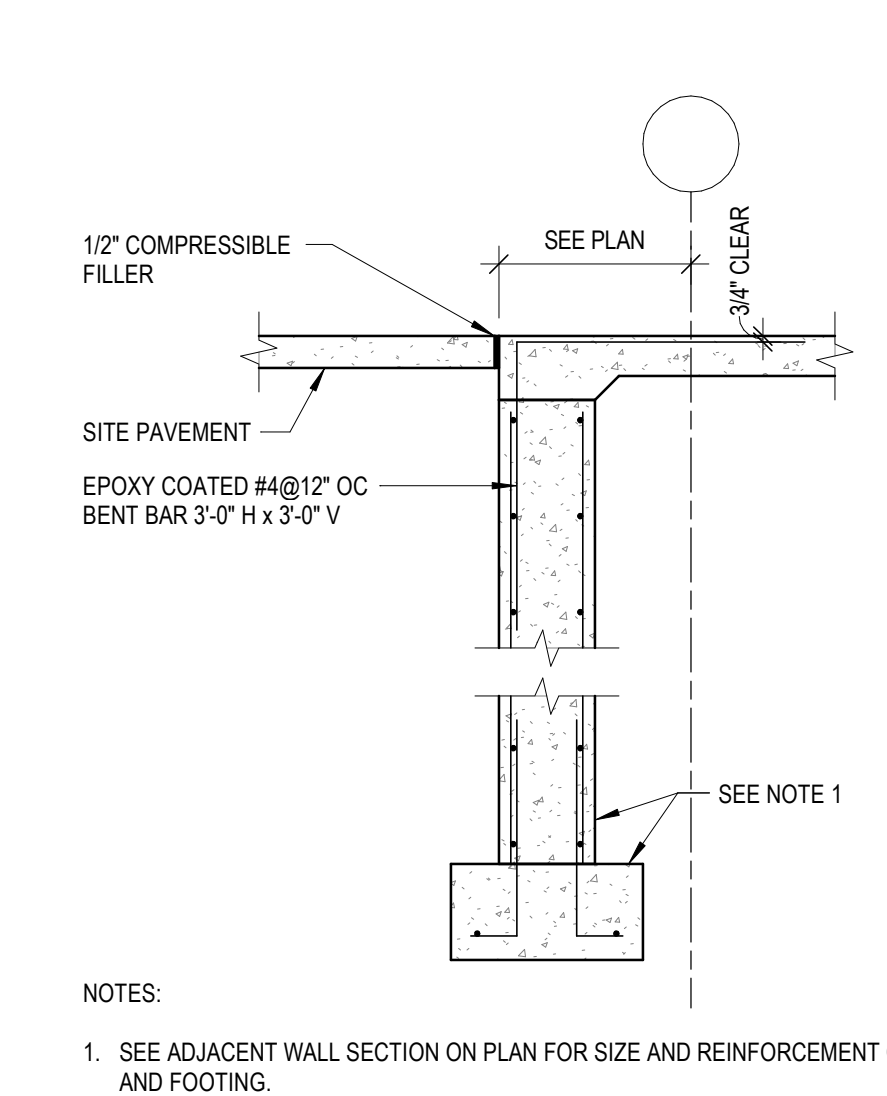
A2 DETAIL
1/2" = 1'-0"



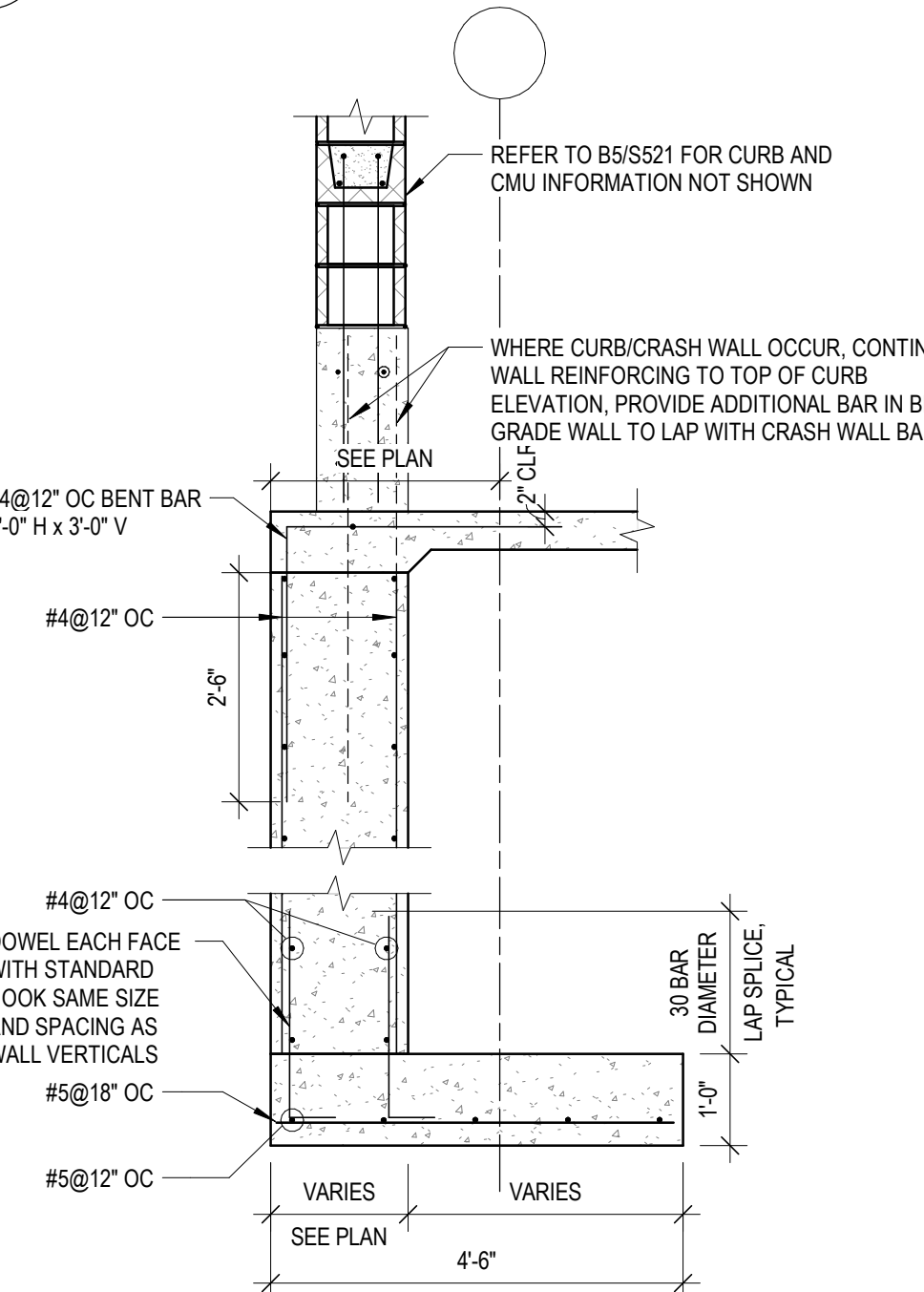
A3 DETAIL
1/2" = 1'-0"



A4 DETAIL
1/2" = 1'-0"



A5 DETAIL
1/2" = 1'-0"



A6 DETAIL
1/2" = 1'-0"

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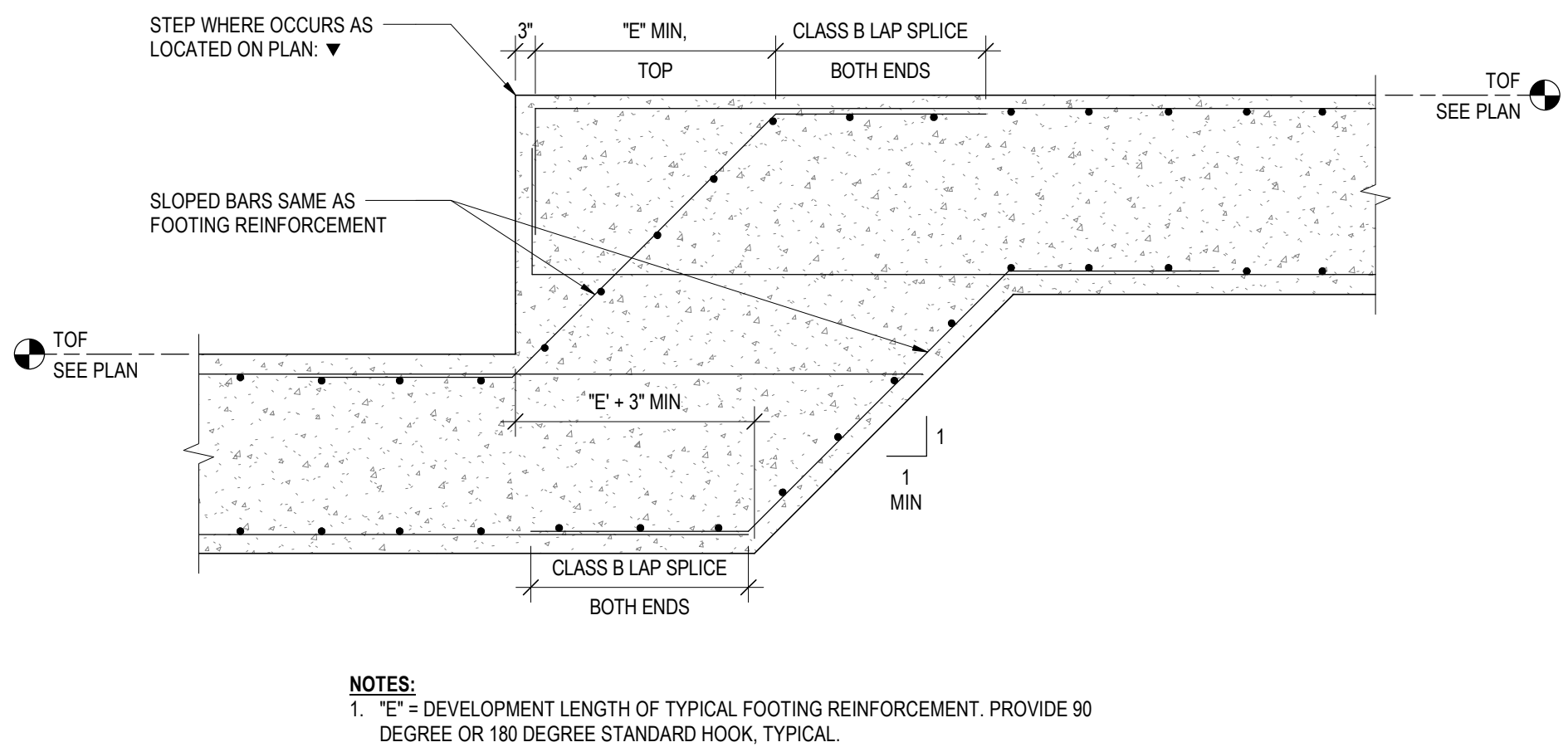
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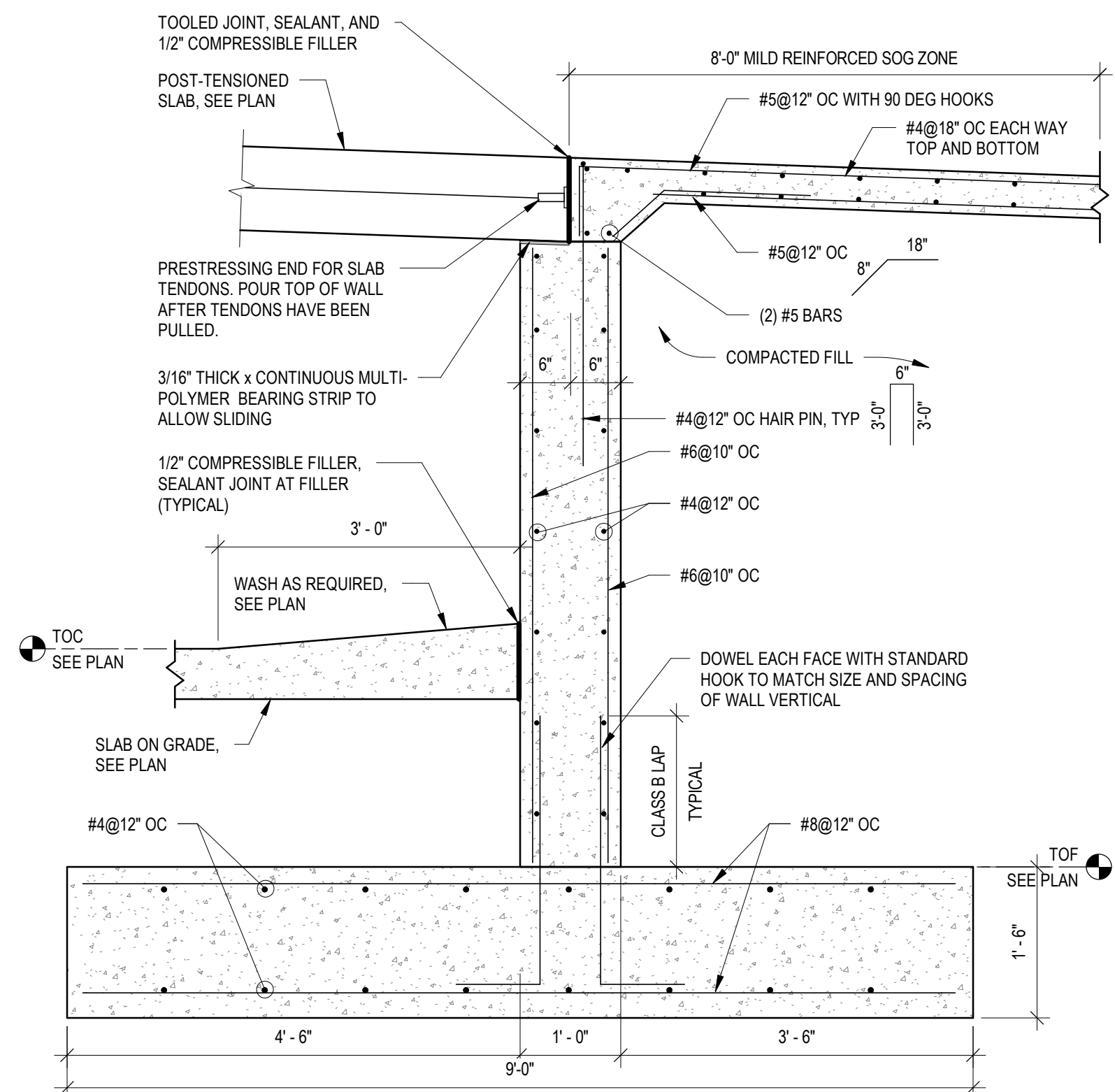
FOUNDATION DETAILS

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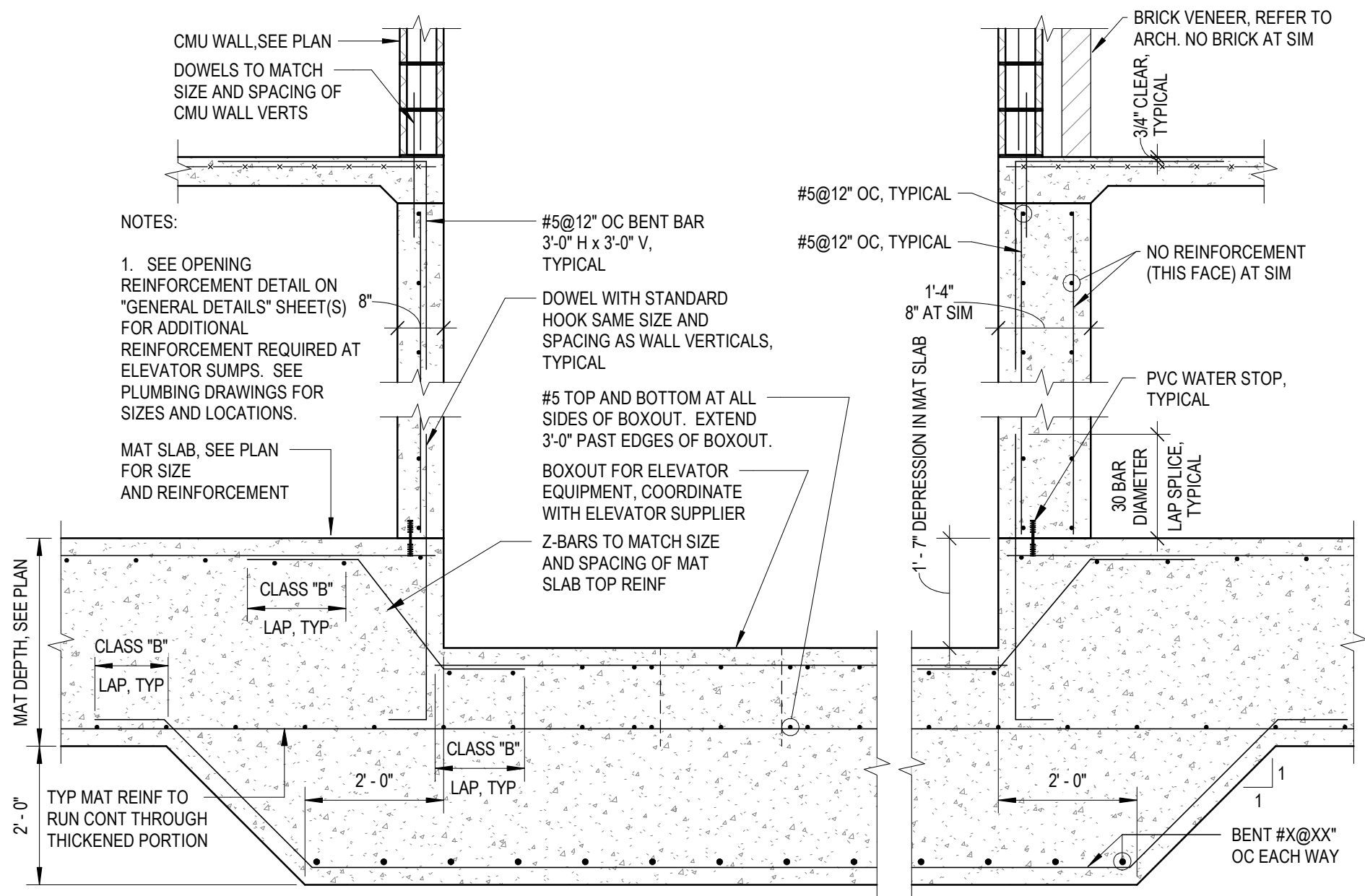
S502



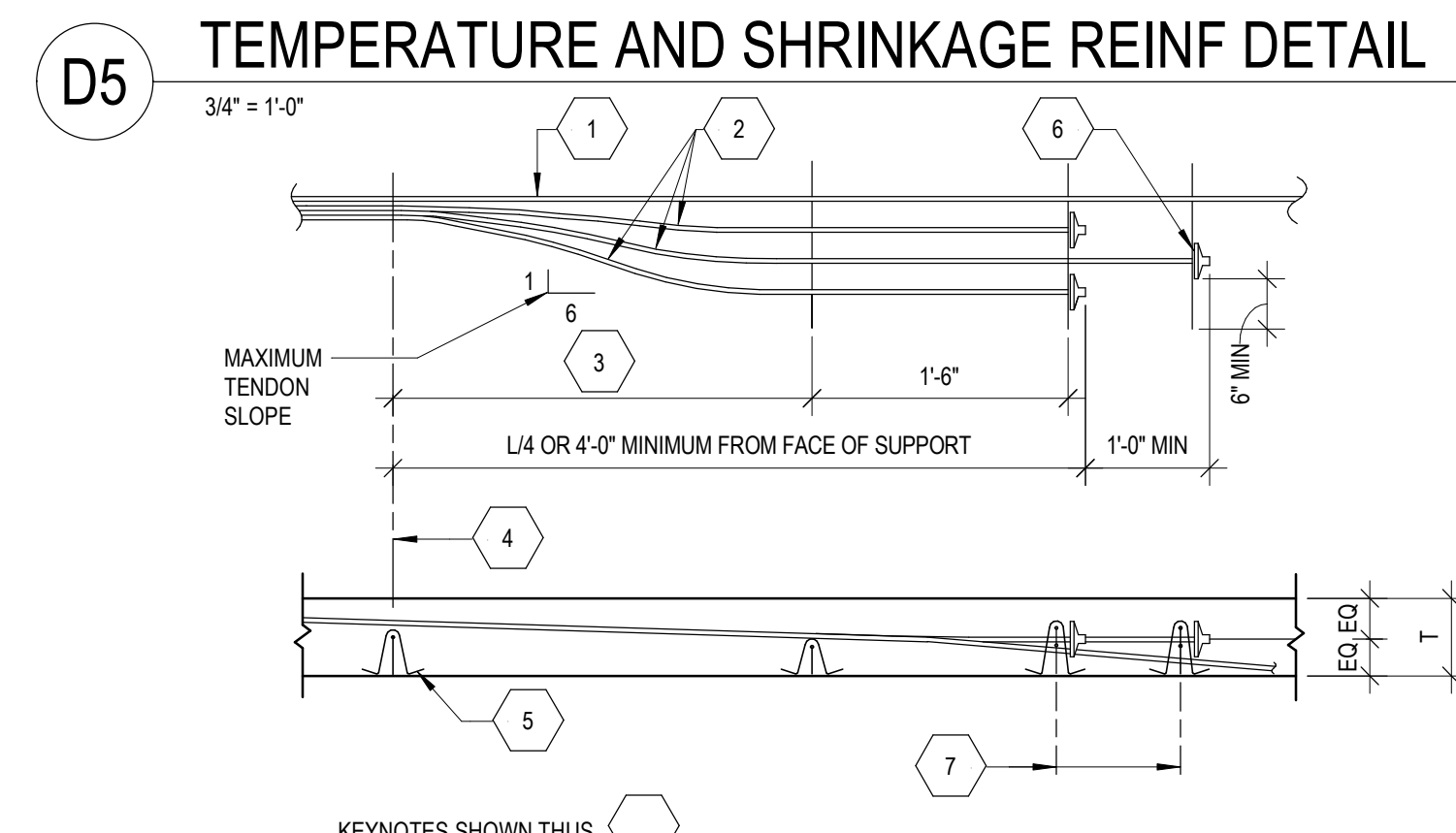
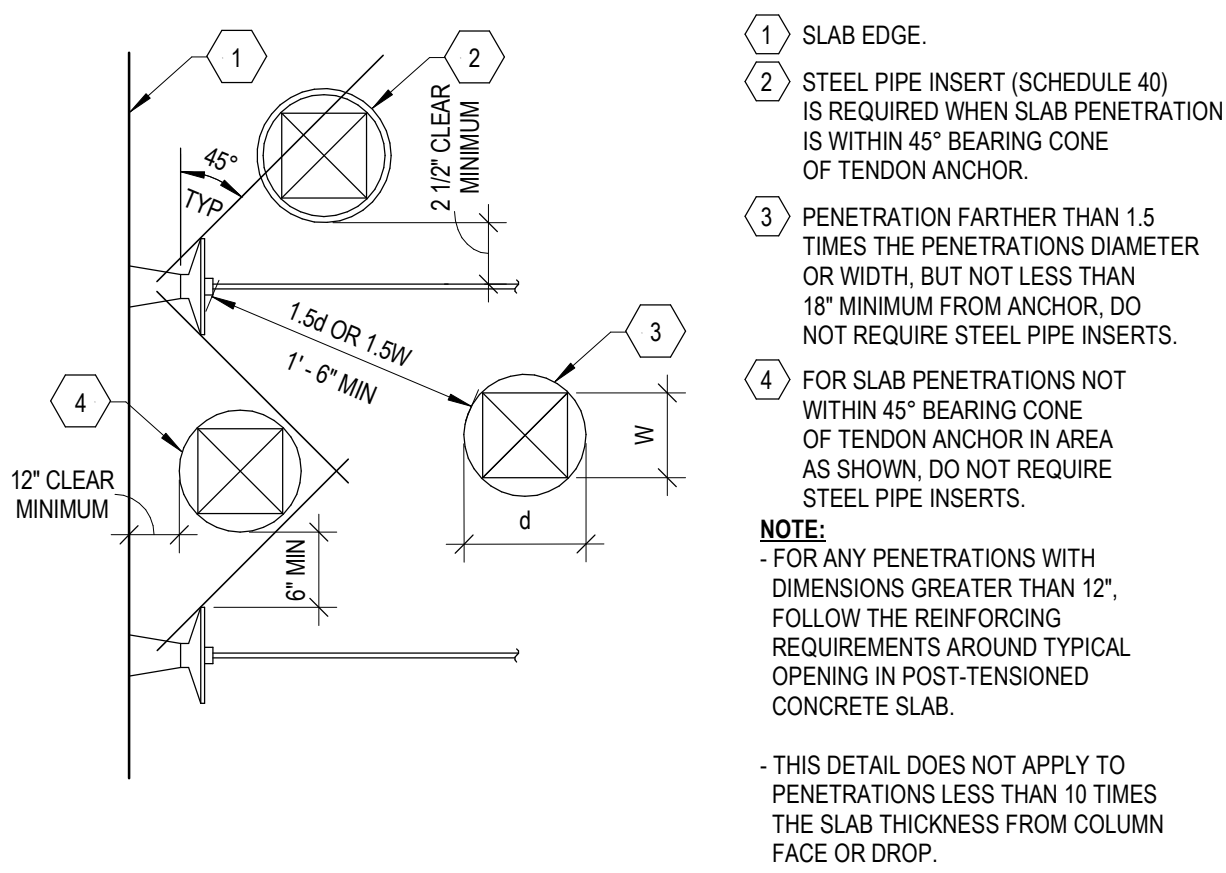
B5 S502-B5
1/2" = 1'-0"



A5 SECTION AT SOG TO PT SLAB TRANSITION
3/4" = 1'-0"

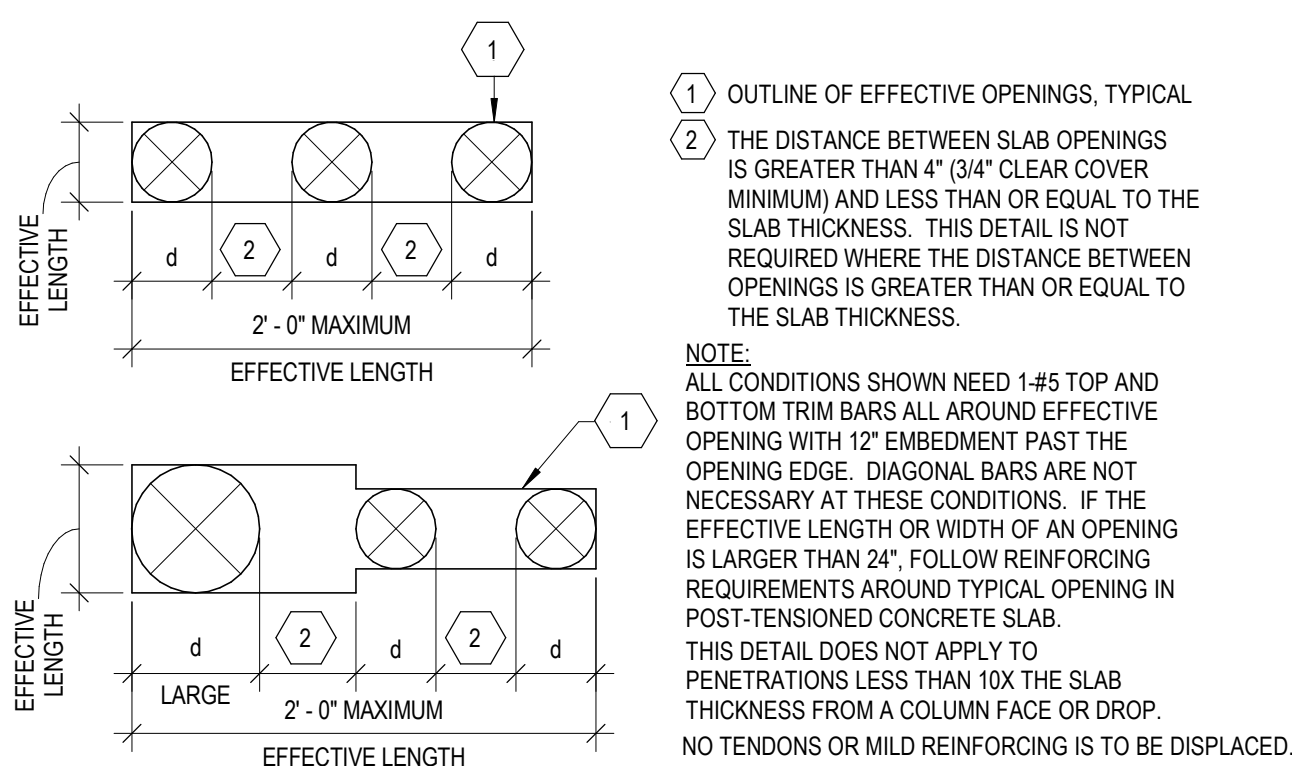


A3 TYPICAL ELEVATOR PIT DETAIL
1/2" = 1'-0"



DEAD END ASSEMBLY

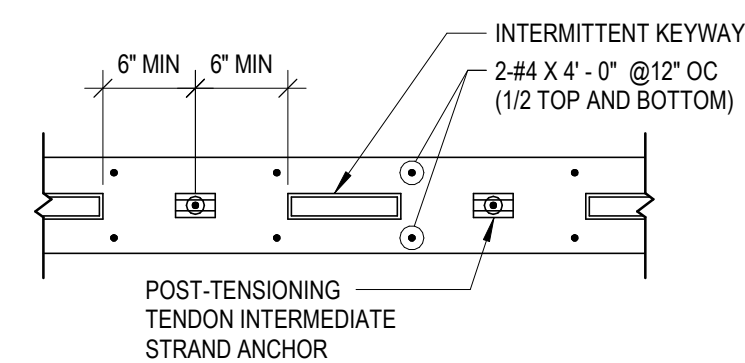
- NOTES:**
1. LOCATE ANCHOR AT BULKHEAD PER APPROVED SHOP DWGS.
 2. IF FABRICATED AT SHOP, WEDGES, GREASE CAP AND SLAB WILL BE ASSEMBLED TIGHT TO ANCHOR.
 3. IF FIELD SEATING IS REQUIRED, BE SURE ALL COMPONENTS ARE TIGHT TO ANCHOR, BE SURE NO BARE STRAND IS EXPOSED. TAPE IF NECESSARY.
 4. APPLY ADDITIONAL PT GREASE PRIOR TO SLIDING THE SLEEVE.
 5. VERIFY ALL HARDWARE DIMENSIONS WITH APPROVED SHOP DWGS.



7. 2-#4 CONTINUOUS MINIMUM BACK-UP BARS (TYPICAL)

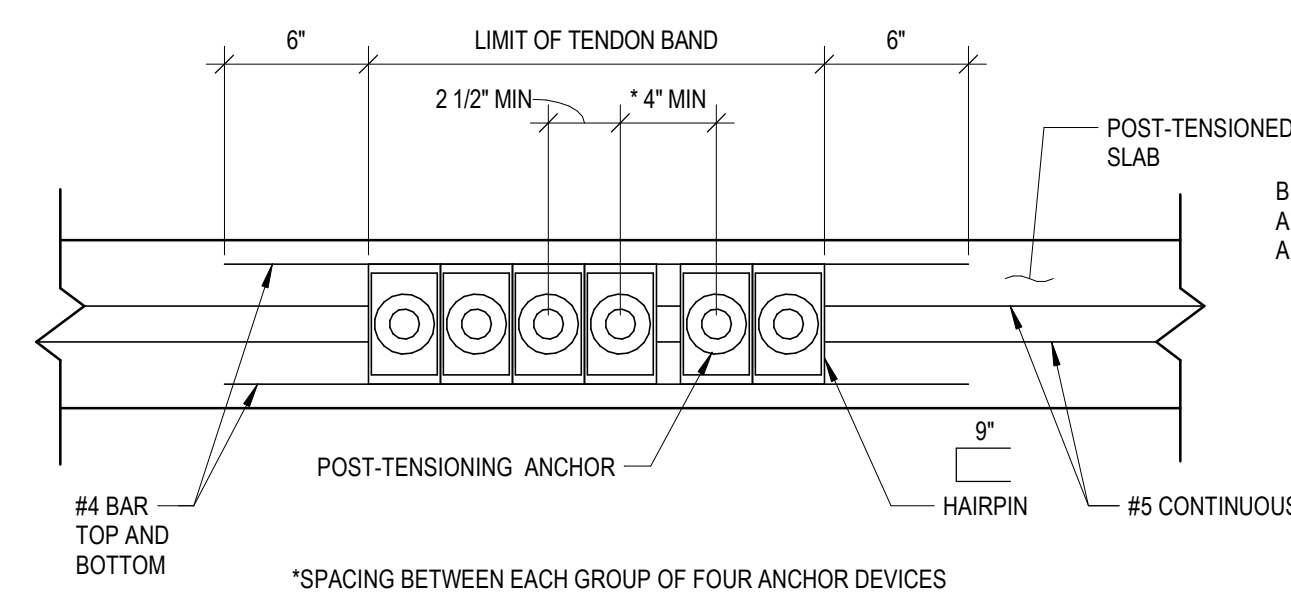
ADDED TENDON ANCHORAGES IN POST TENSION SLAB DETAIL

1. CONTINUOUS TENDON.
2. ADDED TENDONS.
3. SUPPORT BARS @ 48" OC MAXIMUM
4. CENTERLINE OF SUPPORT.
5. CHAIR (TYPICAL)
6. STAGGER DEAD END ANCHORAGES WHEN MORE THAN TWO ARE PLACED IN ONE LOCATION
7. 2-#4 CONTINUOUS MINIMUM BACK-UP BARS (TYPICAL)



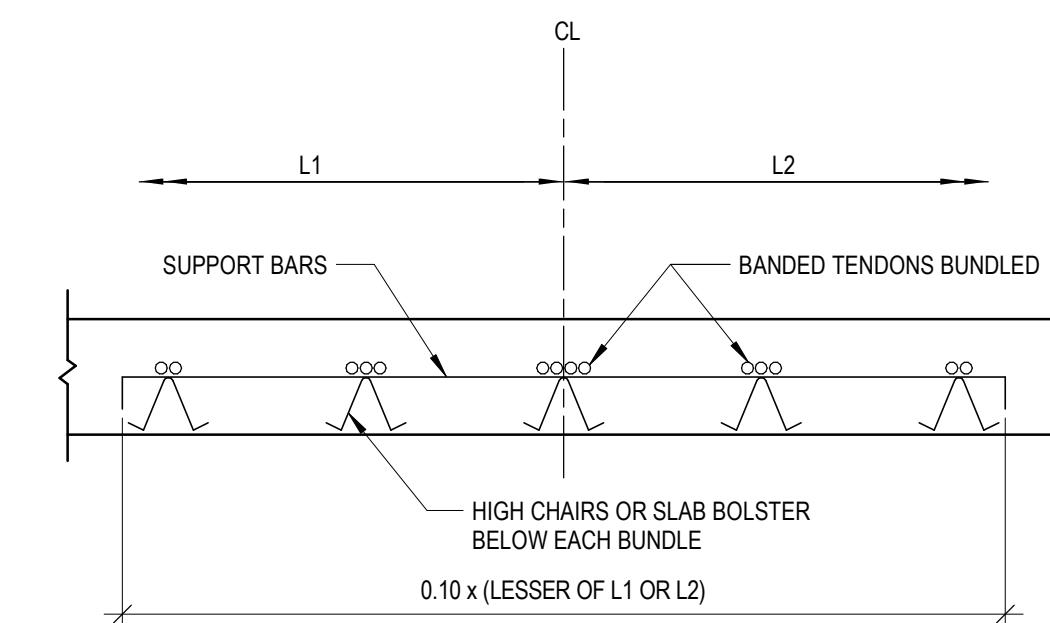
INTERMEDIATE CJ DETAIL

-
- 6" MIN 6" MIN
- INTERMITTENT KEYWAY
- 2#4 X 4'-0" @ 12" OC (1/2 TOP AND BOTTOM)
- POST-TENSIONING TENDON INTERMEDIATE STRAND ANCHOR



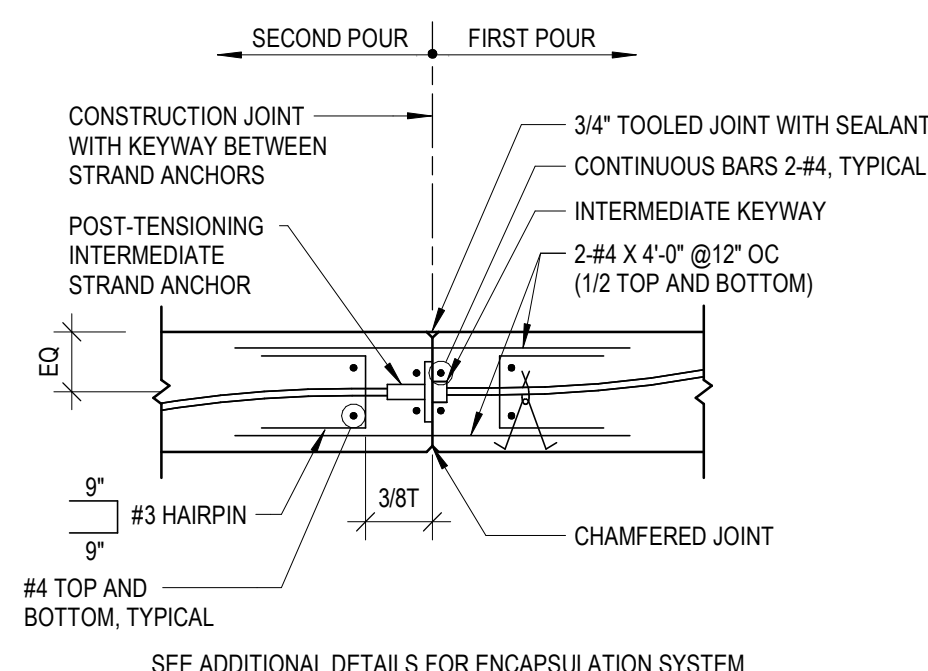
STRESSING END DETAIL

-



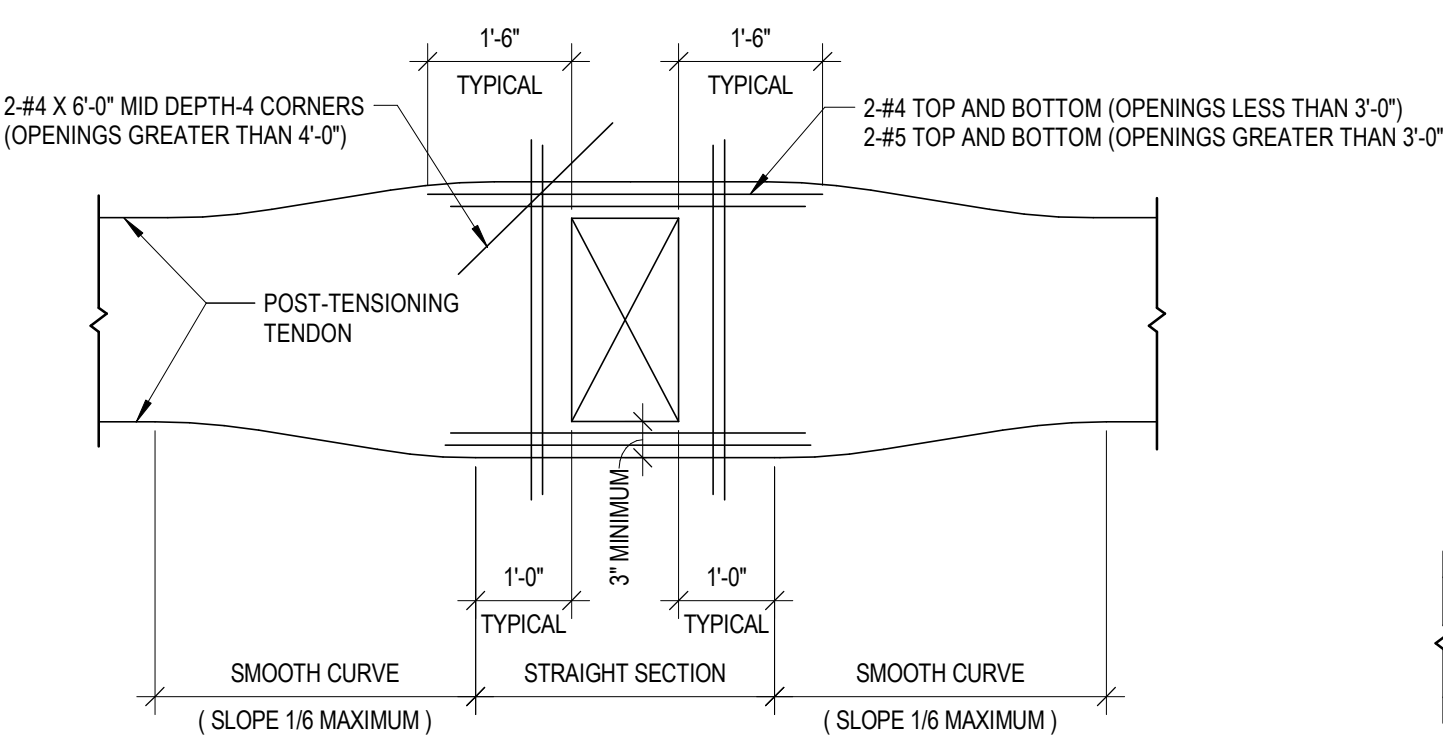
BANTED TENDON GROUP DETAIL

-



INTERMEDIATE STRESSING AT TENDONS DETAIL

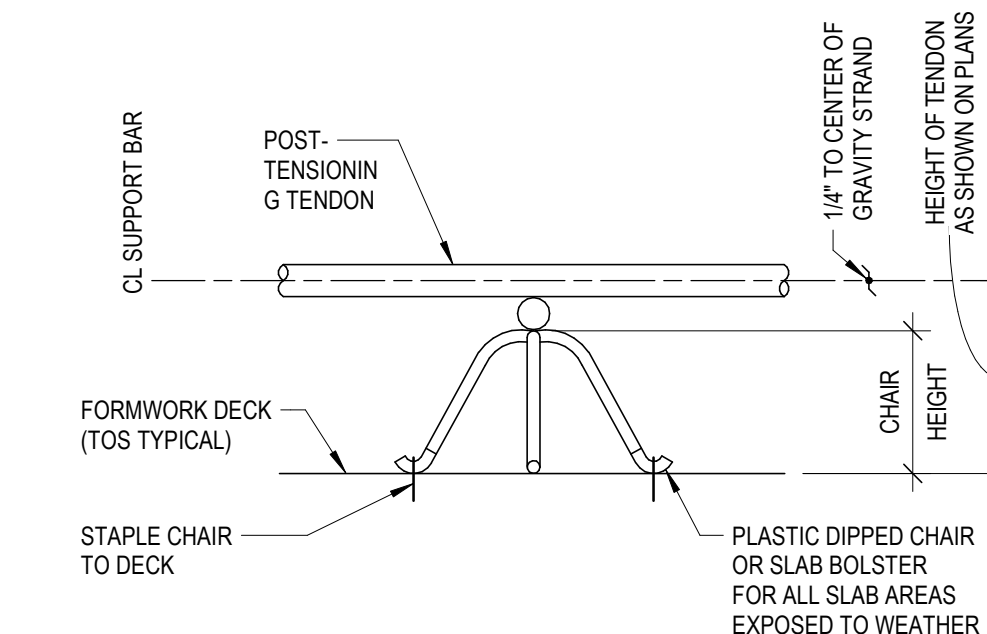
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SEE ADDITIONAL DETAILS FOR ENCAPSULATION SYSTEM

NON STRESSING END DETAIL

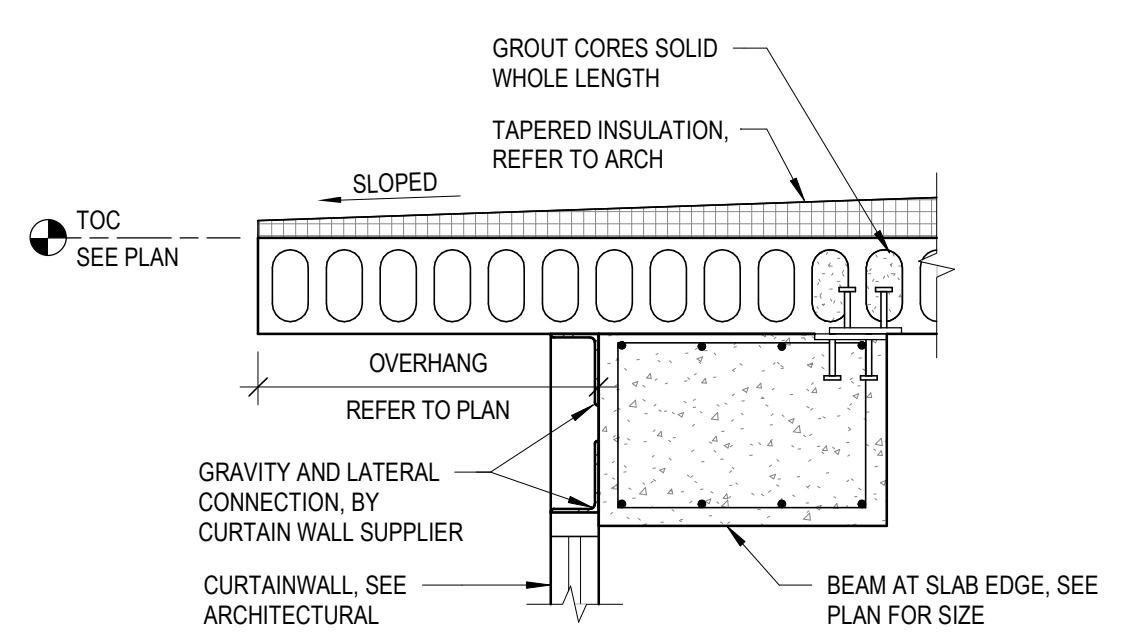
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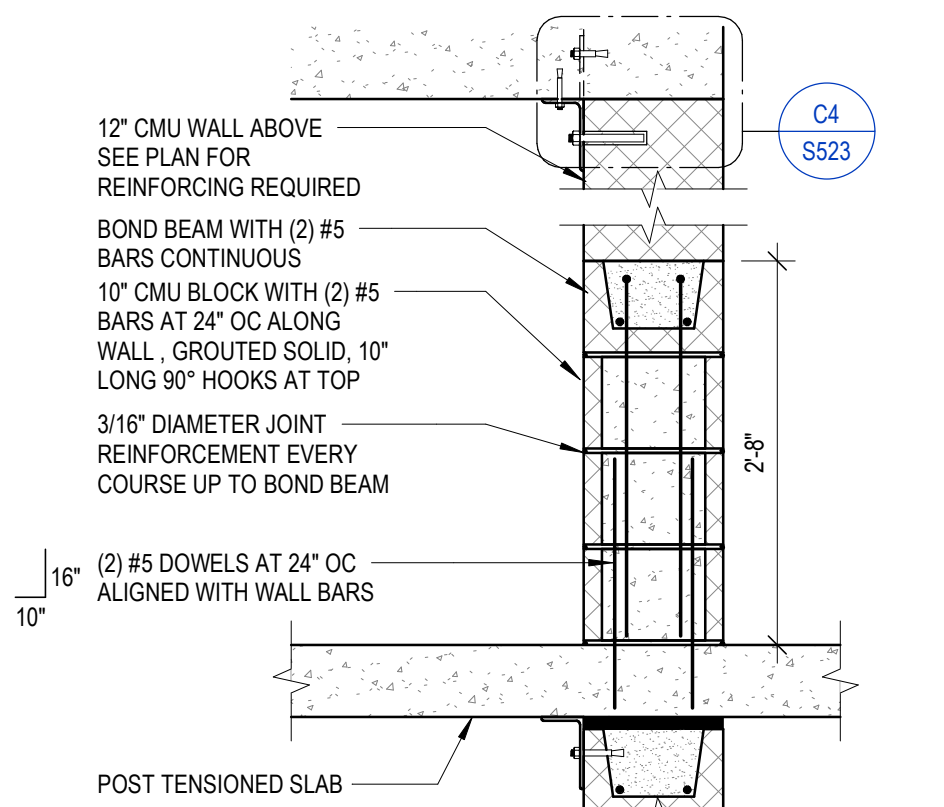
TENDON SUPPORT DETAIL

- [illegible]

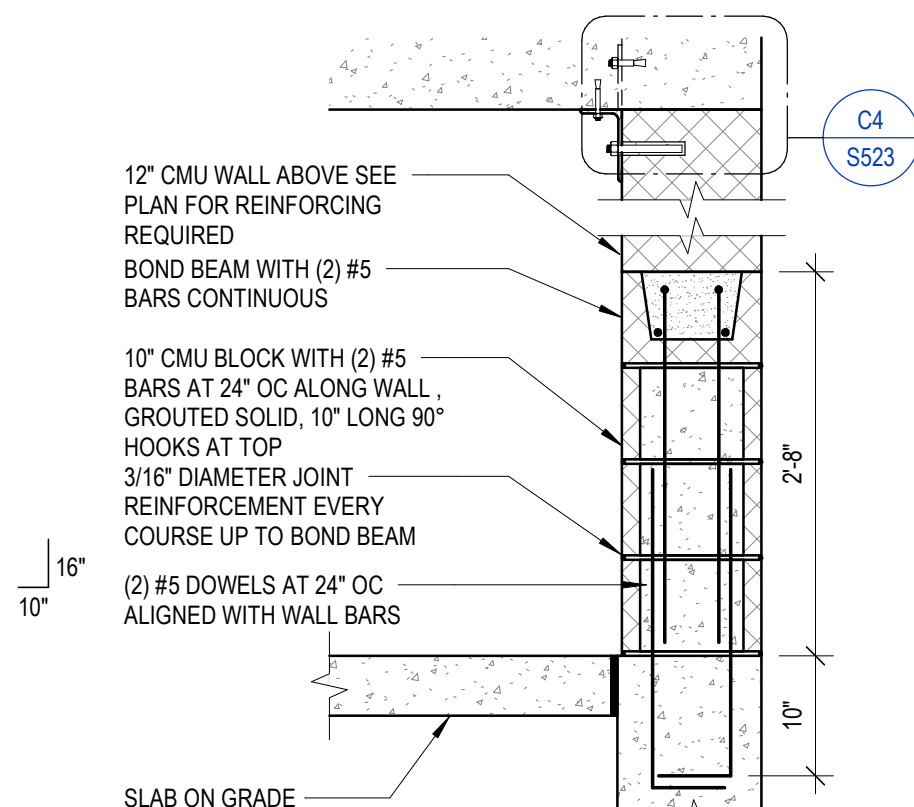




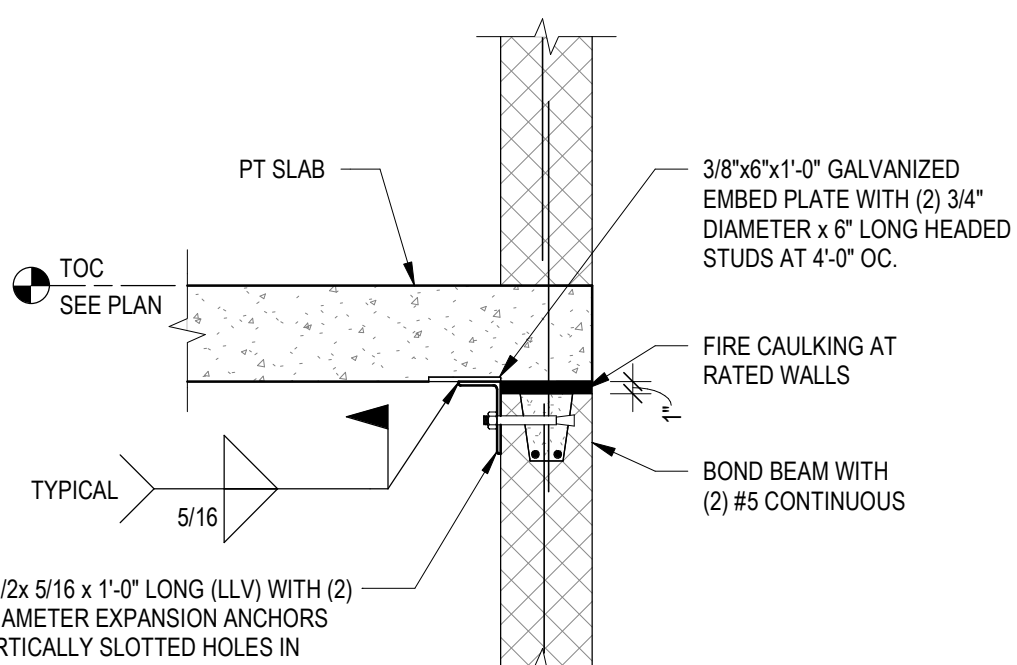
D6
3/4" = 1'-0"



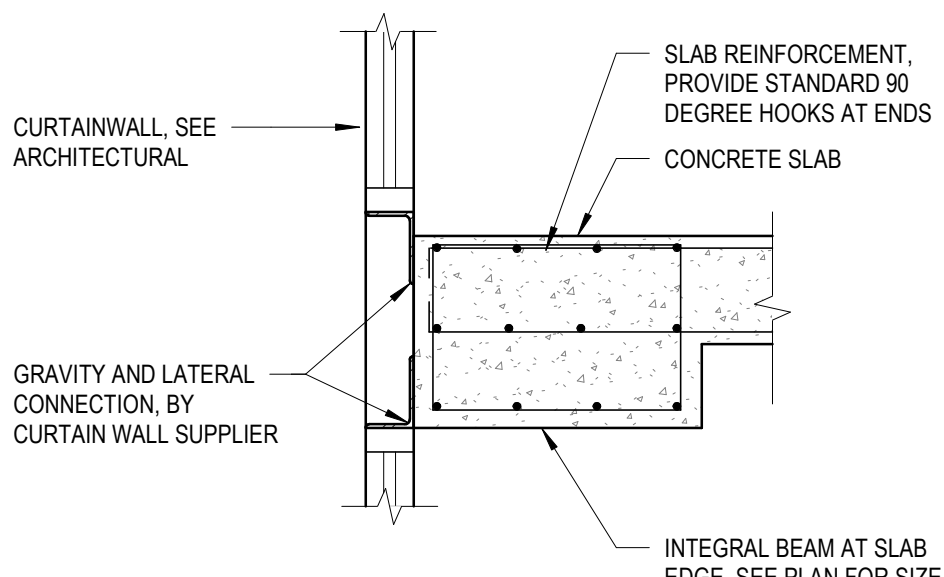
C6
3/4" = 1'-0"



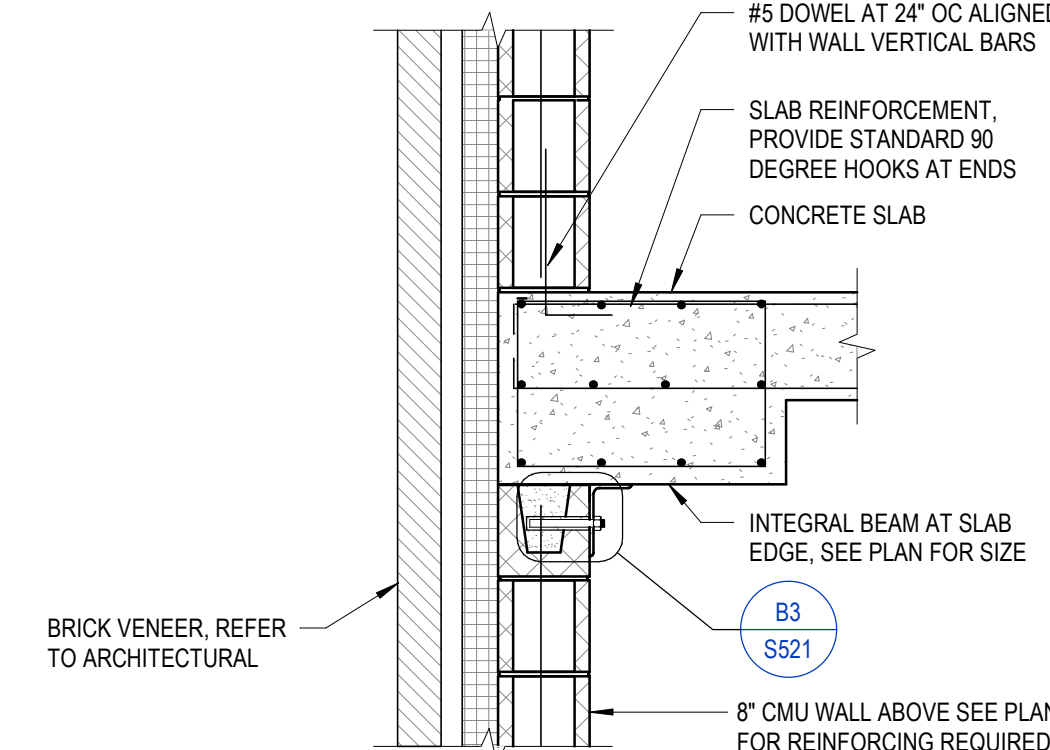
C5
3/4" = 1'-0"



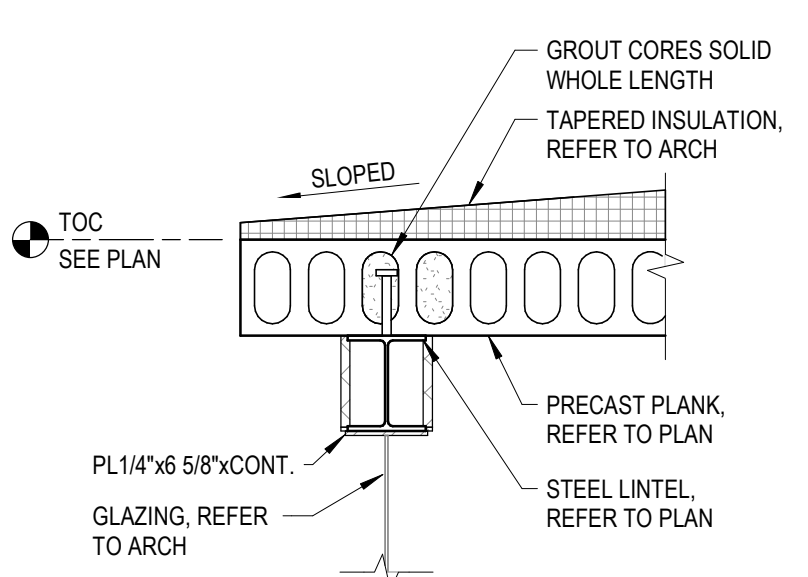
C4
3/4" = 1'-0"



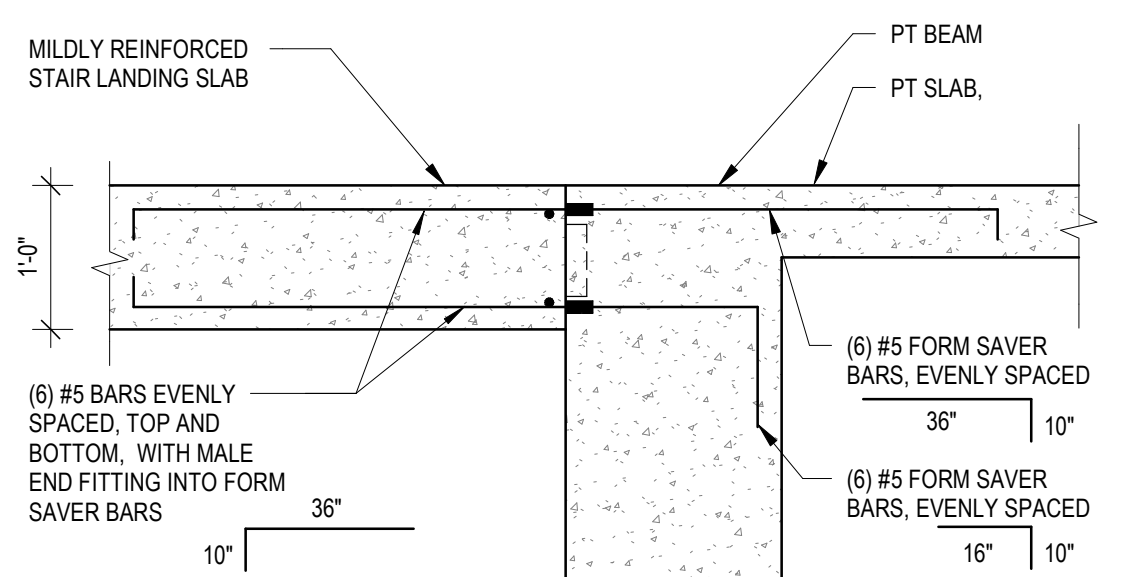
B6
3/4" = 1'-0"



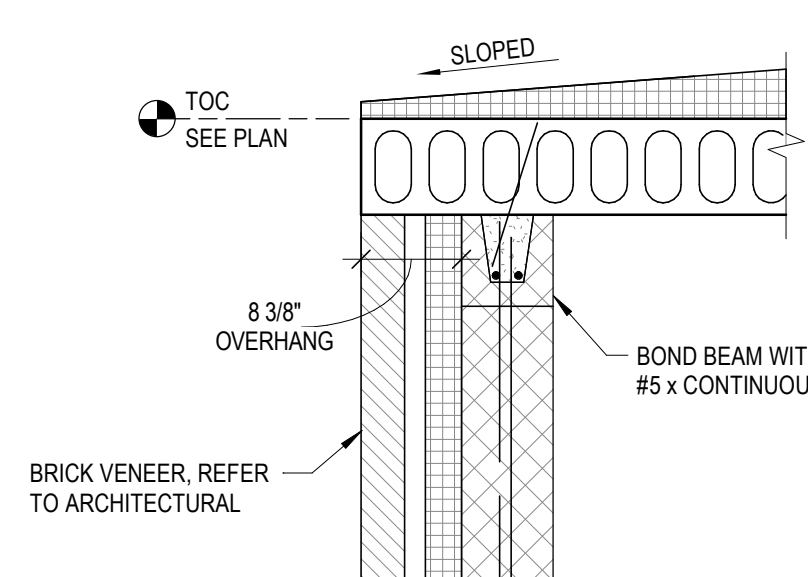
B5
3/4" = 1'-0"



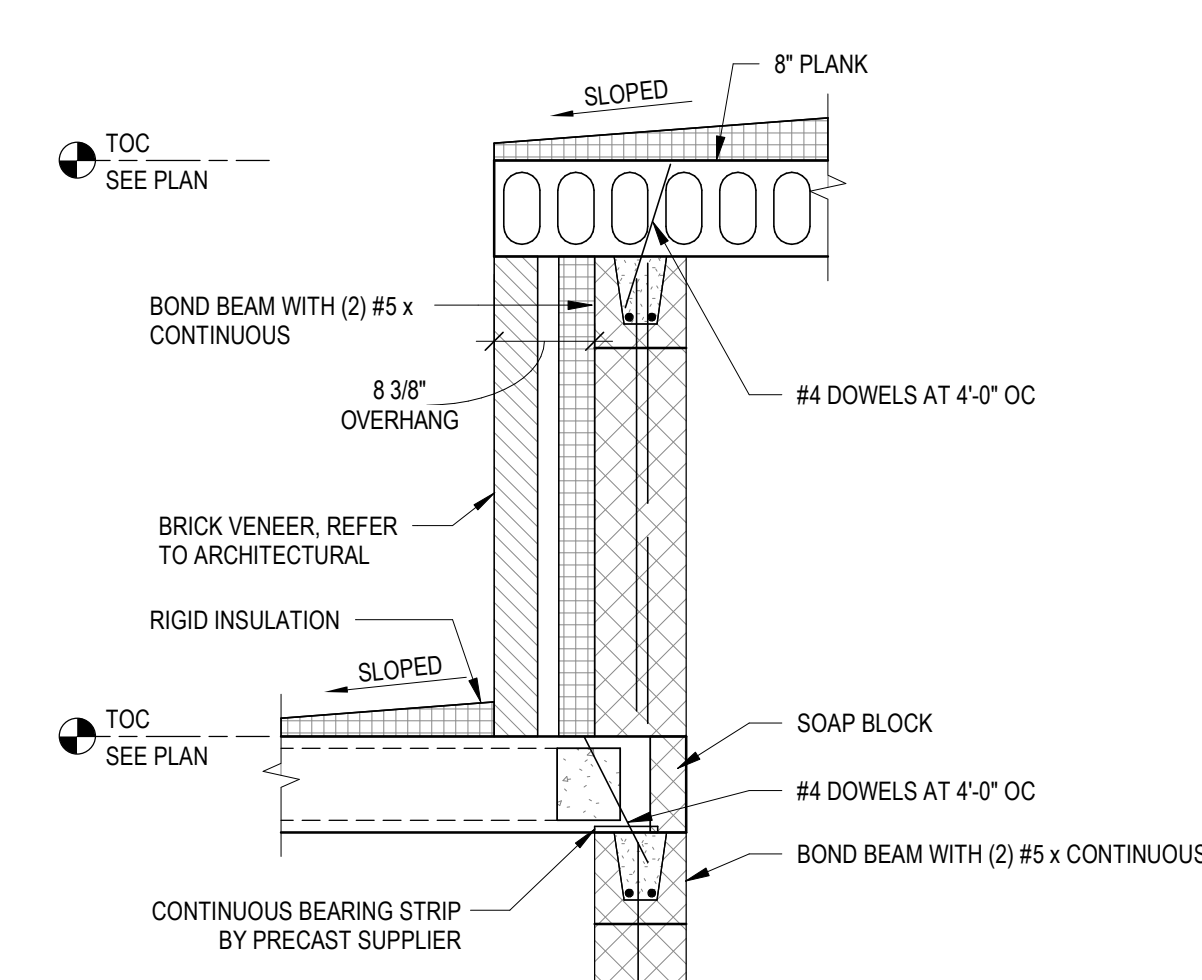
B4
3/4" = 1'-0"



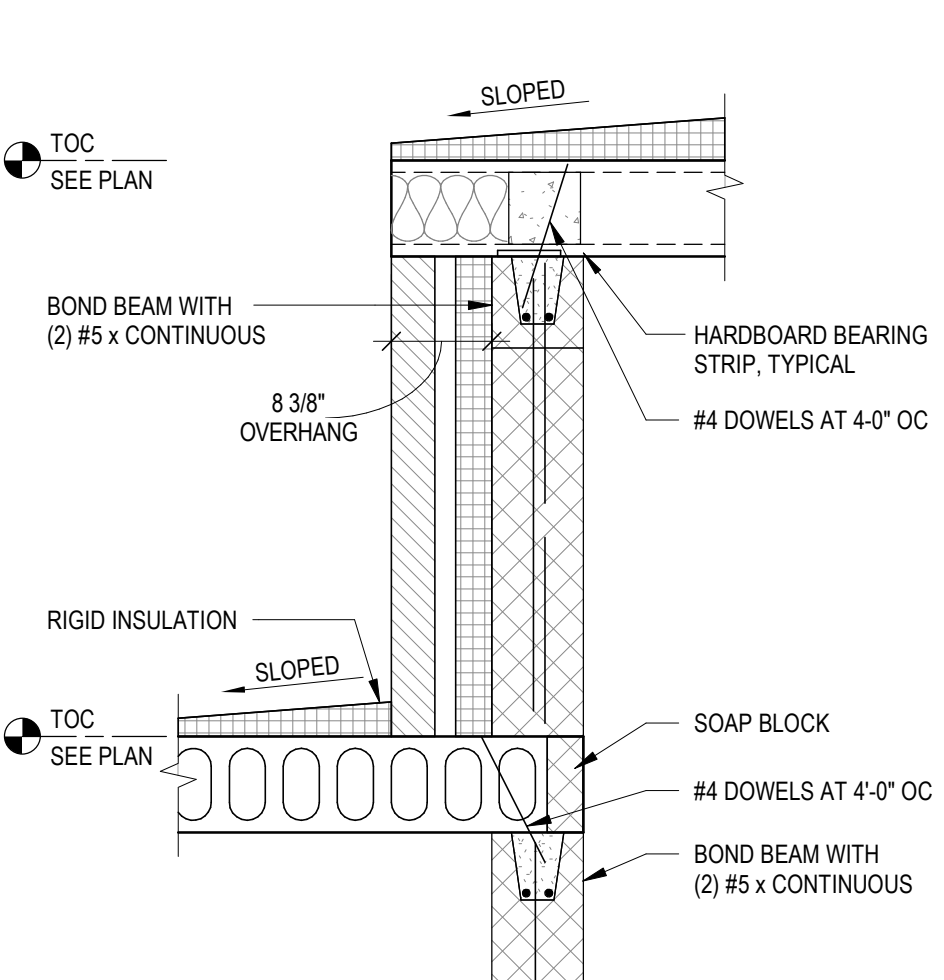
B3
3/4" = 1'-0"



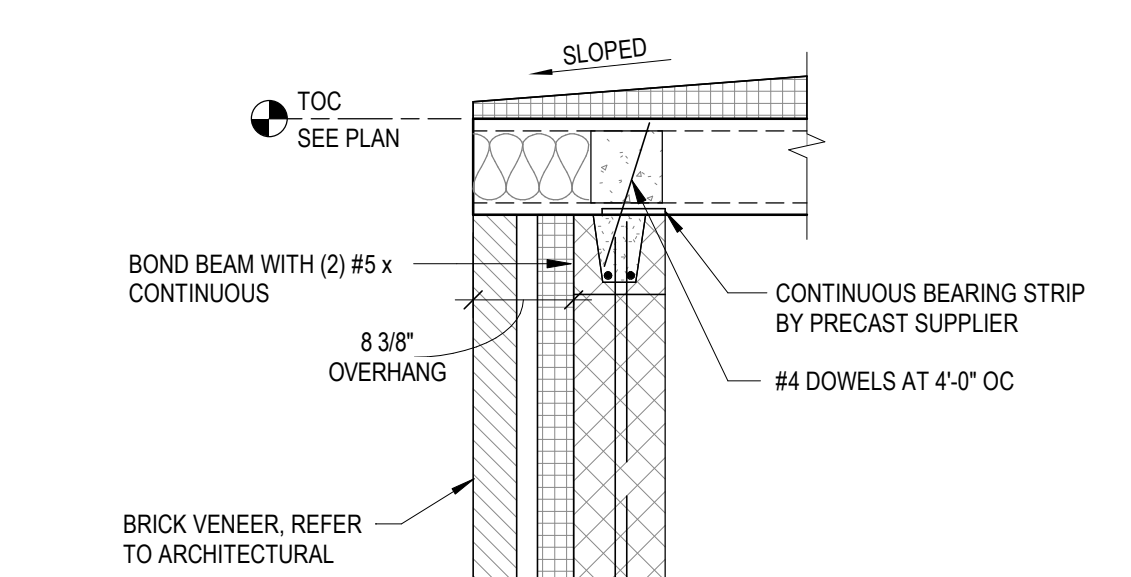
A4
3/4" = 1'-0"



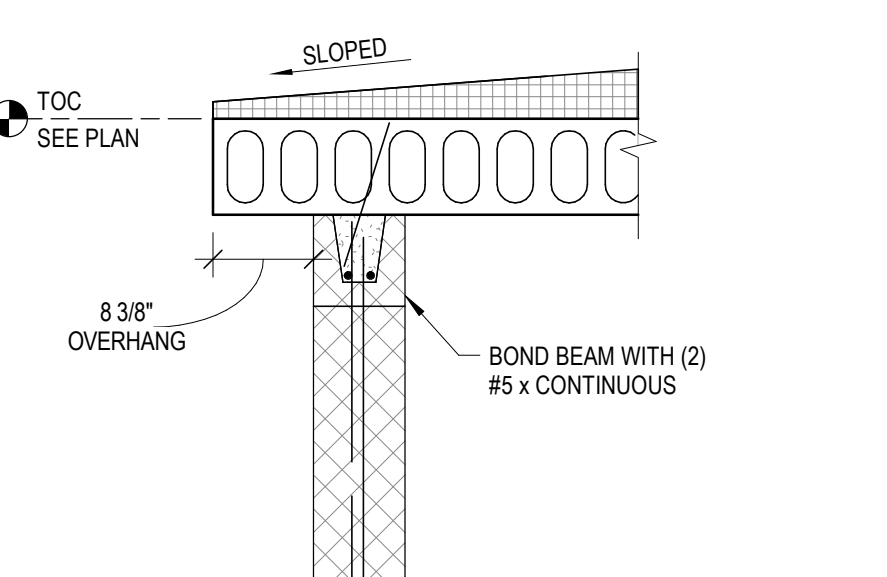
A5
3/4" = 1'-0"



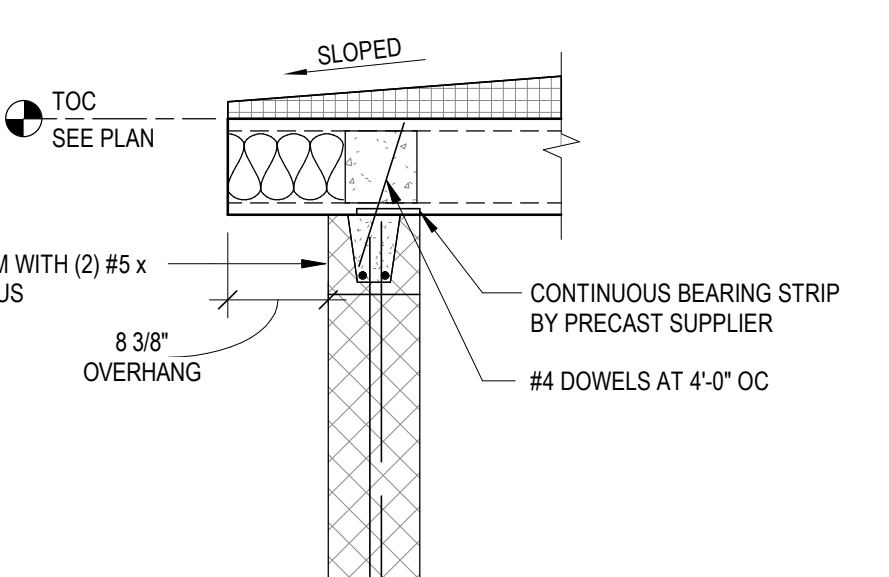
A6
3/4" = 1'-0"



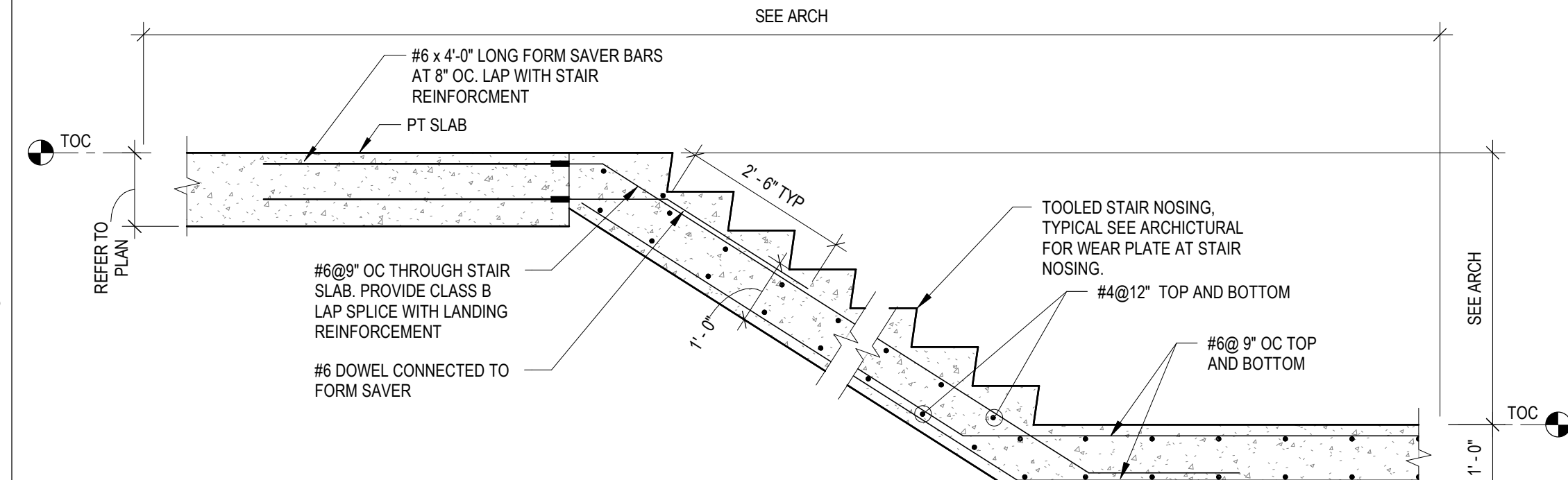
A3
3/4" = 1'-0"



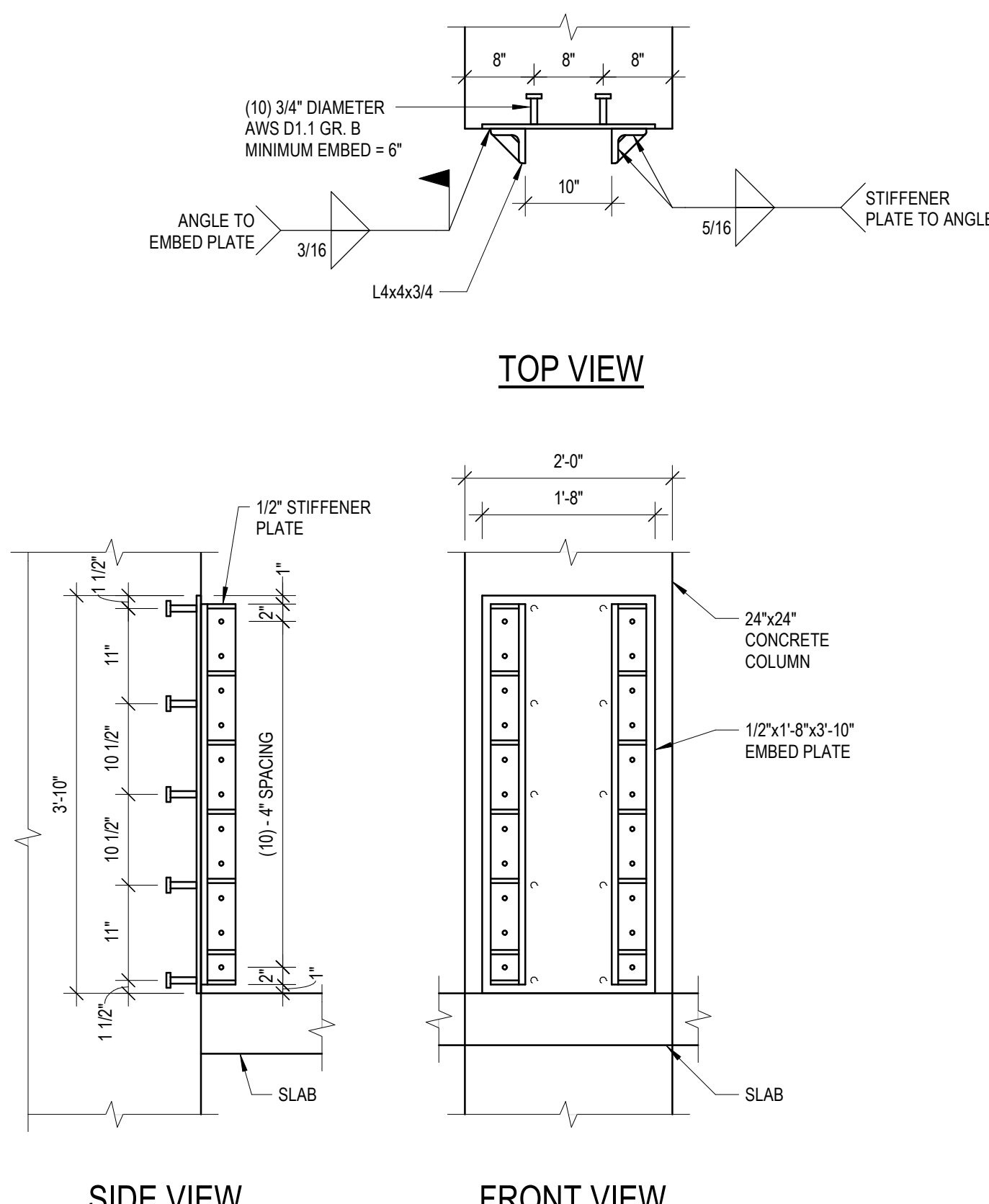
A2
3/4" = 1'-0"



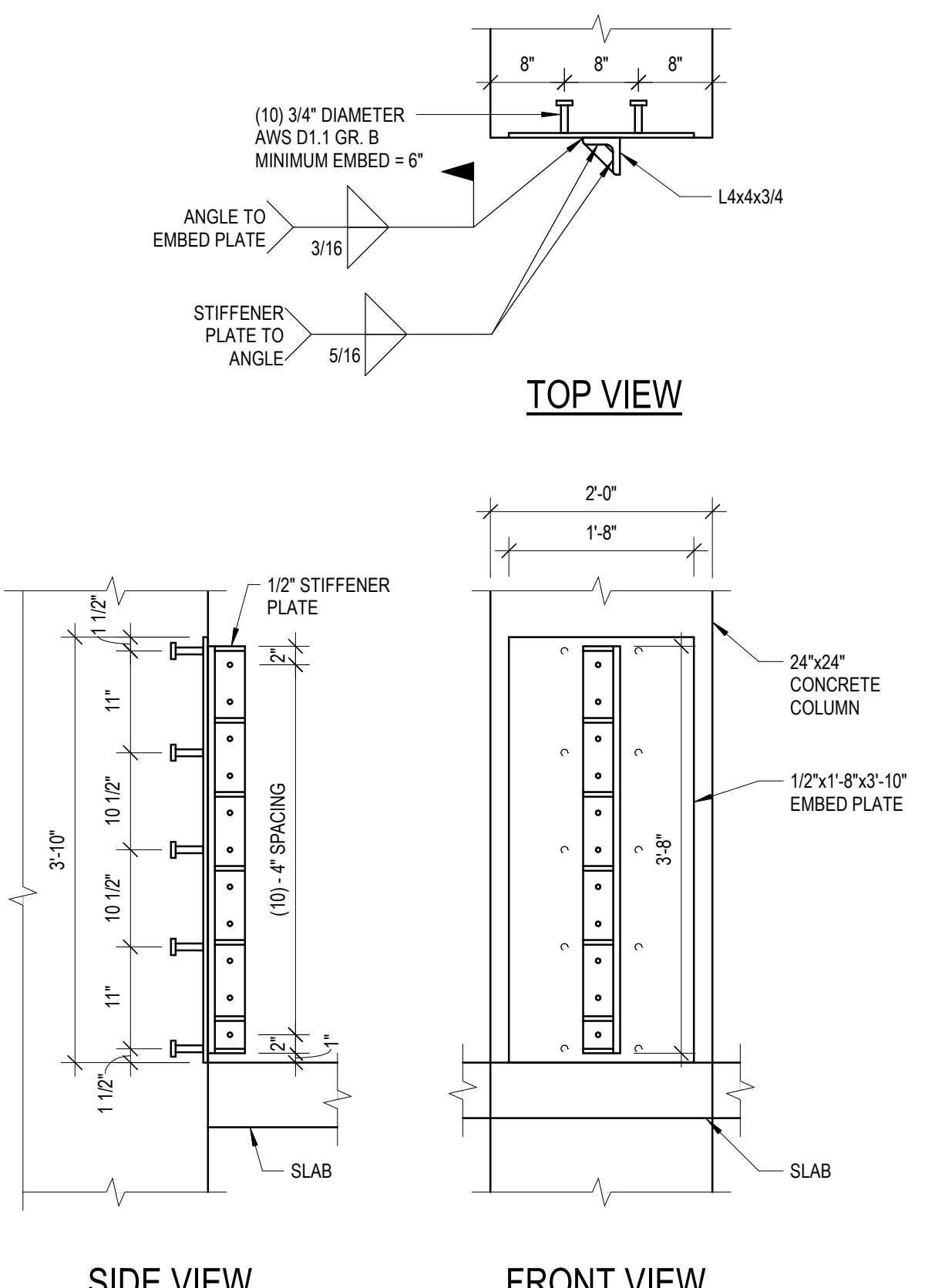
A1
3/4" = 1'-0"



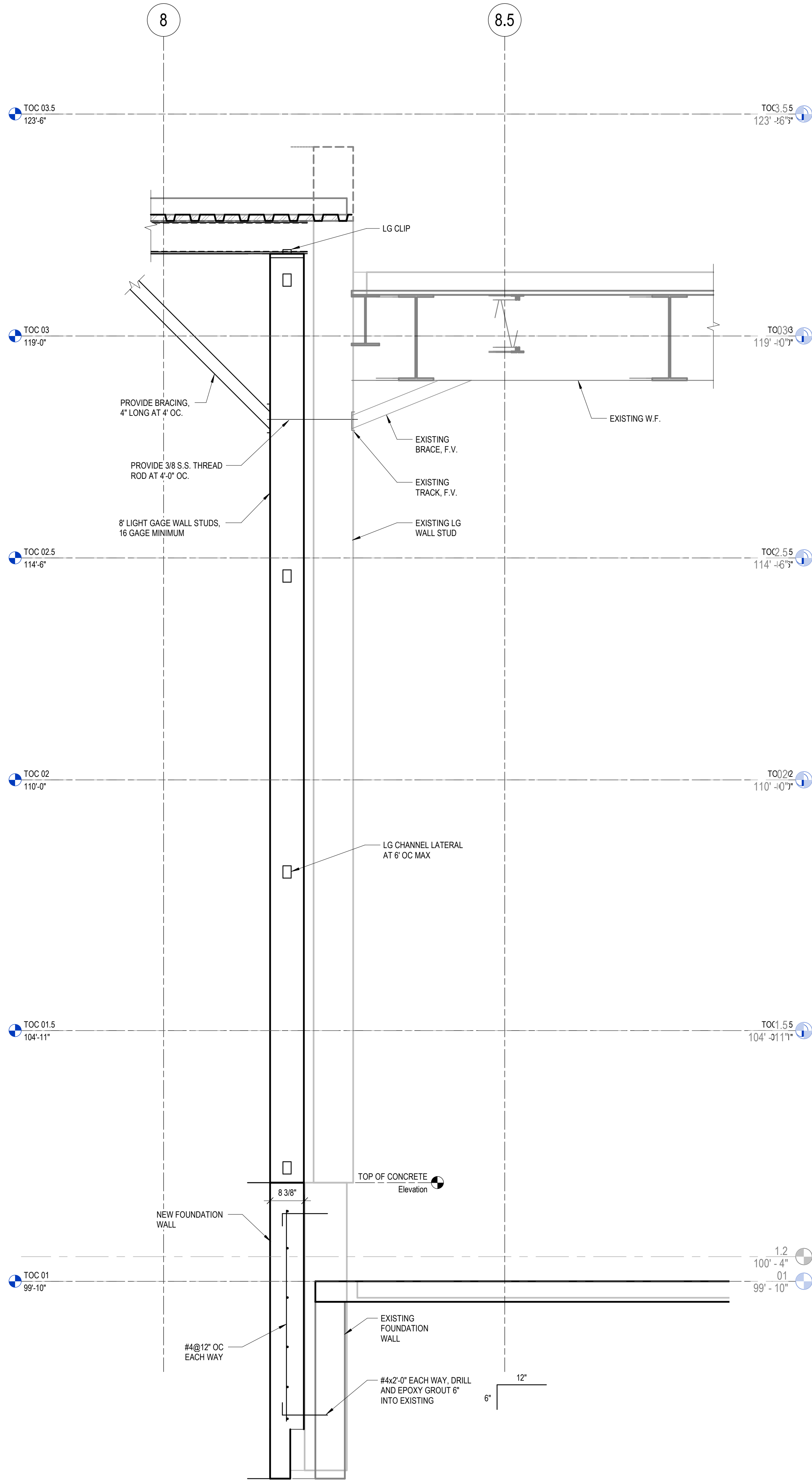
B1
1/2" = 1'-0"



C1
3/4" = 1'-0"



C2
3/4" = 1'-0"



A1 SECTION AT NEW STUD WALL

3/4" = 1'-0"



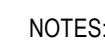
SHEARHEAD SCHEDULE NOTES:

1. FOR LOCATION:
I = INTERIOR, SEE INTERIOR COLUMN
DETAIL THIS SHEET FOR
SHEARHEAD LAYOUT.
E = EXTERIOR, SEE EDGE COLUMN
DETAIL THIS SHEET FOR
SHEARHEAD LAYOUT.
C = CORNER, SEE CORNER COLUMN
DETAIL THIS SHEET FOR
SHEARHEAD LAYOUT.
2. FOR COLUMN TYPE:
S = SQUARE OR RECTANGULAR
C = CIRCULAR
3. FOR SECTION AT STUDDAIL SEE
STUDDAIL DIAGRAM DETAIL THIS SHEET.

CONCRETE COLUMN SCHEDULE						
MARK		C01	C02	C03	C04	C05
LEVEL						
TOCC LEVEL SIX						
SEE PLAN	SIZE (WAL)	24" x 24"	48" x 24"	48" x 24"	16" x 16"	18" x 18"
	VERTICALS	(12) #8	(12) #8	(8) #8	(8) #8	(8) #8
	TIES	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC
	TYPE	IV	IV	II	II	II
TOCC LEVEL FIVE	CONCRETE STRENGTH (PSI)	5000	5000	5000	5000	5000
SEE PLAN	SIZE (WAL)	24" x 24"	48" x 24"	48" x 24"	16" x 16"	18" x 18"
	VERTICALS	(12) #8	(12) #8	(16) #8	(8) #8	(8) #8
	TIES	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC
	TYPE	IV	IV	II	II	II
TOCC LEVEL FOUR	CONCRETE STRENGTH (PSI)	5000	5000	5000	5000	5000
SEE PLAN	SIZE (WAL)	24" x 24"	48" x 24"	48" x 24"	16" x 16"	18" x 18"
	VERTICALS	(12) #8	(16) #8	(16) #8	(8) #8	(8) #8
	TIES	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC
	TYPE	IV	IV	II	II	II
TOCC LEVEL THREE	CONCRETE STRENGTH (PSI)	5000	5000	5000	5000	5000
SEE PLAN	SIZE (WAL)	24" x 24"	48" x 24"	48" x 24"	16" x 16"	18" x 18"
	VERTICALS	(12) #8	(16) #8	(16) #8	(8) #8	(8) #8
	TIES	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC
	TYPE	IV	IV	II	II	II
TOCC LEVEL TWO	CONCRETE STRENGTH (PSI)	5000	5000	5000	5000	5000
SEE PLAN	SIZE (WAL)	24" x 24"	48" x 24"	48" x 24"	16" x 16"	18" x 18"
	VERTICALS	(12) #8	(16) #8	(16) #8	(8) #8	(8) #8
	TIES	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC
	TYPE	IV	IV	II	II	II
TOCC LEVEL ONE	CONCRETE STRENGTH (PSI)	5000	5000	5000	5000	5000
SEE PLAN	SIZE (WAL)	24" x 24"	48" x 24"	48" x 24"	16" x 16"	18" x 18"
	VERTICALS	(12) #8	(16) #8	(16) #8	(8) #8	(8) #8
	TIES	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC	#4@8" OC
	TYPE	IV	IV	II	II	II
TOF	CONCRETE STRENGTH (PSI)	5000	5000	5000	5000	5000
SEE PLAN						

CONCRETE COLUMN SCHEDULE NOTES:

1. SEE TYPICAL COLUMN DIAGRAM DETAIL THIS SHEET FOR INFORMATION PERTAINING TO COLUMN SCHEDULE.
2. COLUMN REINFORCING TO BE CONTINUOUS OR TENSION-SPLICED IN ACCORDANCE WITH ACI 318-14, SECTION 25.5.7 BY WELDED OR MECHANICAL SPLICES. SPLICES OF ADJACENT BARS SHALL BE STAGGERED A MINIMUM OF 2'-6". LAP SPLICES IN COLUMNS SHALL NOT BE PERMITTED.

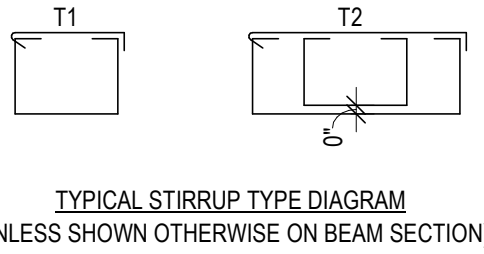
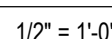


1. STAGGER HOOK LOCATIONS FOR SUCCESSIVE CIRCULAR TIES. ADJACENT CIRCULAR TIES SHOULD NOT ENGAGE THE SAME LOGITUDINAL BAR

$$1/2'' = 1'-0''$$

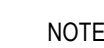
CONCRETE BEAM SCHEDULE NOTES:

1. BEAM BOTTOM BAR SUPPORTS SHALL BE IN ACCORDANCE WITH ACI DETAILING MANUAL. (PLASTIC TIPPED).
2. PLACING TOLERANCES FOR ALL REINFORCEMENT TO CONFORM TO CRSI RECOMMENDED PRACTICES FOR PLACING REINFORCEMENT BARS.
3. SEE TYPICAL BEAM DIAGRAM THIS SHEET FOR INFORMATION PERTAINING TO BEAM SCHEDULE UNLESS NOTED OTHERWISE IN SCHEDULE.
4. SEE TYPICAL INTERIOR AND EXTERIOR BEAM DETAILS THIS SHEET FOR ADDITIONAL REQUIREMENTS.
5. SEE TYPICAL BEAM CONSTRUCTION JOINT DETAIL THIS SHEET FOR BEAM CONSTRUCTION JOINTS.
6. PROVIDE INDIVIDUAL SHOP DRAWING PLANS AND SCHEDULES FOR ALL FLOORS.
7. WHEN CONTINUOUS TOP BARS ARE INDICATED IN SCHEDULE PROVIDE CLASS B TENSION LAP SPICE AT CENTERLINE OF SPAN OF BEAM.
8. CAMBER PER SPECIFICATIONS UNLESS NOTED IN SCHEDULE.
9. BEAMS OVER OPENINGS ARE MINIMUM, NOT NECESSARILY THE REQUIRED HEIGHT OF OPENINGS. CONTRACTOR TO COORDINATE.

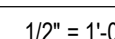
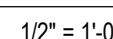
 $1/2'' = 1'-0''$ 

NOTE: PROVIDE HOLES IN EMBED PLATES TO ACCOMMODATE LIGHT POLE ANCHOR BOLTS. PROVIDE SLOTTED HOLE TO ACCOMMODATE EMBEDDED CONDUIT. COORDINATE SIZES AND LOCATIONS WITH ELECTRICAL SUBCONTRACTOR.

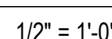
3/4" = 1'-0"



1. PROVIDE TWO TIES WITHIN SLAB DEPTH AT EXTERIOR COLUMN.
2. PROVIDE TIES AT 6" OC ALONG SLOPE.

$$1/2^s =$$


A3



1/2" = 1'-0"

3/4" = 1'-0"

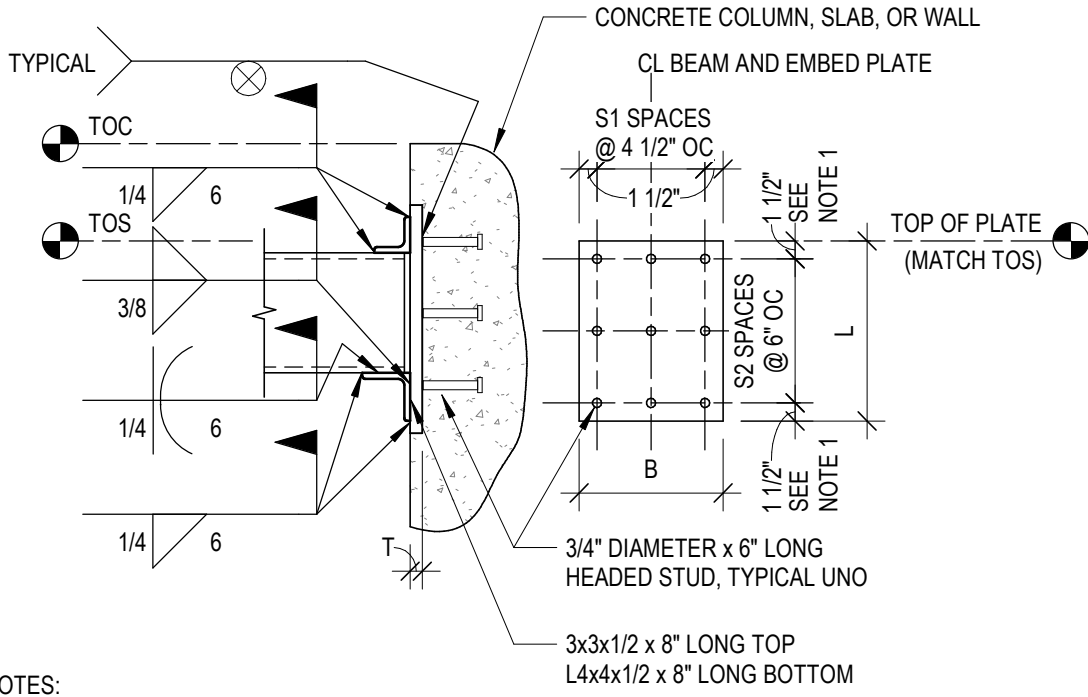
$$1/2^s =$$

SHEET NUMBER:

S601

Autodesk Docs://20225013.00 - Village on Park Garage/20225013_S22.rvt
7/25/2023 3:29:02 PM

EMBED PLATE SCHEDULE						
MARK	PLATE SIZE (TxB)	S1	S2	TOP OF PLATE ELEVATION	DETAIL REFERENCE	REMARKS
EP01	SEE DETAIL	2	2	TOCC. SEE PLANS	AS/S01	GALVANIZED - FUTURE PV FRAMING ATTACHMENTS. SEE DETAIL FOR S1, S2 SPACING
EP02	1/2"x1-10"x12"	2	3	146'-9"	N/A	GALVANIZED
EP03	1/2"x1-7"x9"	2	2	MATCH TOC	N/A	GALVANIZED. GC TO COORDINATE FINAL LOCATIONS WITH POST TENSIONING. SEE NOTE 2
EP04	1/2"x10"x10"	1	1	MATCH TOC	N/A	STAINLESS
EP05	1"x28"x12"	2	3	MATCH TOC	EMBEDDED PLATE AT SNOW CHUTE DIAGRAM	GALVANIZED. SEE SNOW CHUTE SECTION. VERTICAL EDGE DISTANCE = 5" WITH 14" LONG HWS
EP06	1/2"x1-8"x3'-10"	1	4	SEE DETAILS	CU/S23	GALVANIZED. REFER TO DETAIL FOR S1, S2 SPACING
EP07	3/4"x1-3"x7'-1/2"	1	2	110'-5"	N/A	GALVANIZED. FINAL LOCATION TO BE COORDINATED WITH CANOPY SUPPLIER
EP08	1/2"x12"xCONT	3	AS REQ	TOB, UNO	N/A	GALVANIZED. CONTINUOUS. SEE ENLARGED PLANS AND NOTE 2
EP09	1/2"x9"x9"	1	1	EDGE OF SLAB	A1/S22	STAINLESS. SEE NOTE 2



NOTES:

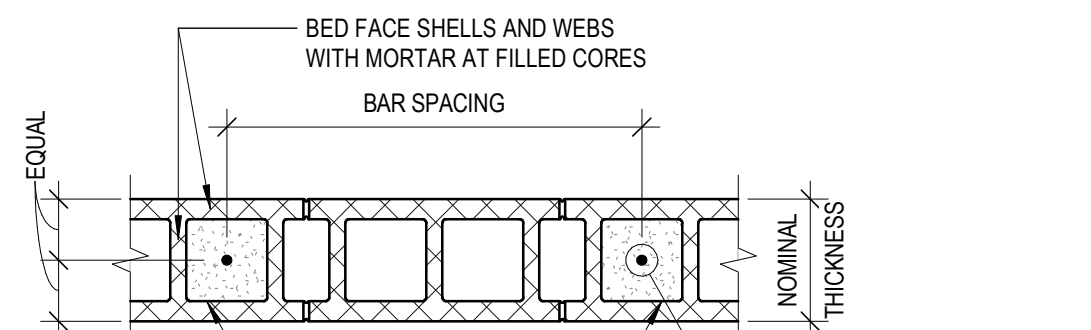
1. INCREASE DIMENSION TO 2" AND INCREASE "L" DIMENSION OF EMBED PLATE AS REQUIRED WHERE EMBED DEPTH MATCHES SLAB OR BEAM THICKNESS.
2. REDUCE S1 SPACING TO 3" WITH 1/2" DIA HEADED STUDS. INCREASE VERTICAL EDGE DISTANCE AS REQUIRED.

EMBED PLATE CONNECTION DIAGRAM

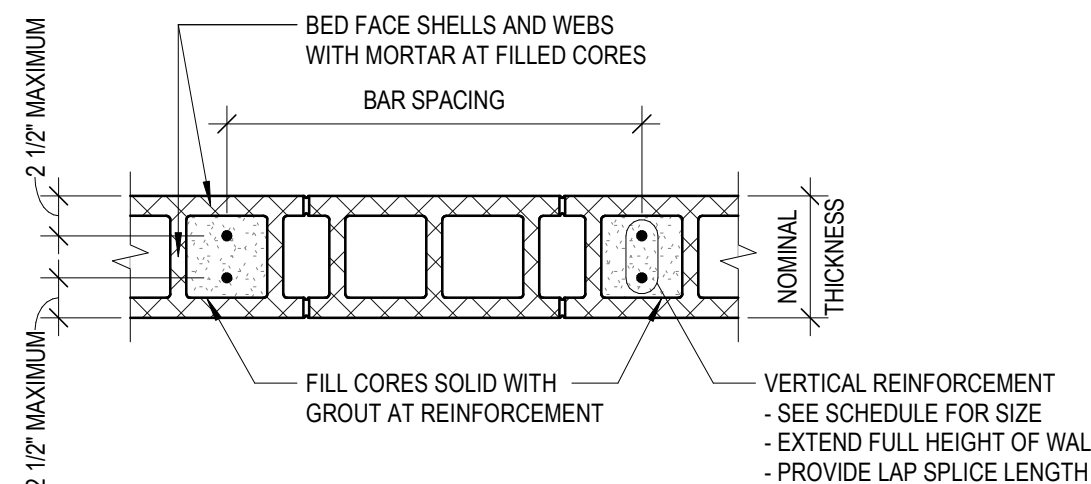
MASONRY WALL SCHEDULE					
MARK	NOMINAL THICKNESS	TYPE	VERTICAL REINFORCEMENT		REMARKS
MM1	8"	A	#5@8" OC		
MM2	10"	B	#5@16" OC		

MASONRY WALL SCHEDULE NOTES:

1. PROVIDE TYPICAL VERTICAL REINFORCEMENT AT WALL ENDS AND EACH SIDE OF CONTROL JOINTS. REINFORCE FIRST TWO CELLS EACH SIDE OF OPENINGS FULL HEIGHT OF WALL. WHERE THE USE OF STEEL OR PRECAST LINTELS INTERRUPTS VERTICAL CONTINUITY OF WALL REINFORCEMENT, SHIFT REINFORCED CELLS PAST LINTEL BEARING AND GROUT WALL SOLID BELOW ENDS OF LINTELS.
2. PROVIDE DOWELS FOR VERTICAL REINFORCEMENT INTO FOUNDATION WALLS AND FOOTINGS BELOW PER DETAILS.
3. SEE GENERAL NOTES AND DETAILS FOR HORIZONTAL JOINT REINFORCEMENT AND BOND BEAM REQUIREMENTS.
4. UNLESS DETAILED OR OTHERWISE CALLED OUT, PROVIDE CMU LINTELS PER LINTEL SCHEDULE OVER OPENINGS IN MASONRY WALLS.
5. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCEMENT IN ALL WALLS AS PER SPECIFICATIONS.
6. OPENINGS IN WALLS PROVIDED FOR MECHANICAL DUCTWORK SHALL BE CENTERED IN BETWEEN BEAM BEARING LOCATIONS OR POSITIONED WITH THE NEAREST EDGE NO CLOSER THAN 24" EITHER SIDE OF BEAM BEARING LOCATIONS.
7. SEE PLAN AND DETAILS FOR ADDITIONAL WALL REINFORCEMENT AND GROUTING REQUIREMENTS NOT COVERED IN THIS SCHEDULE.

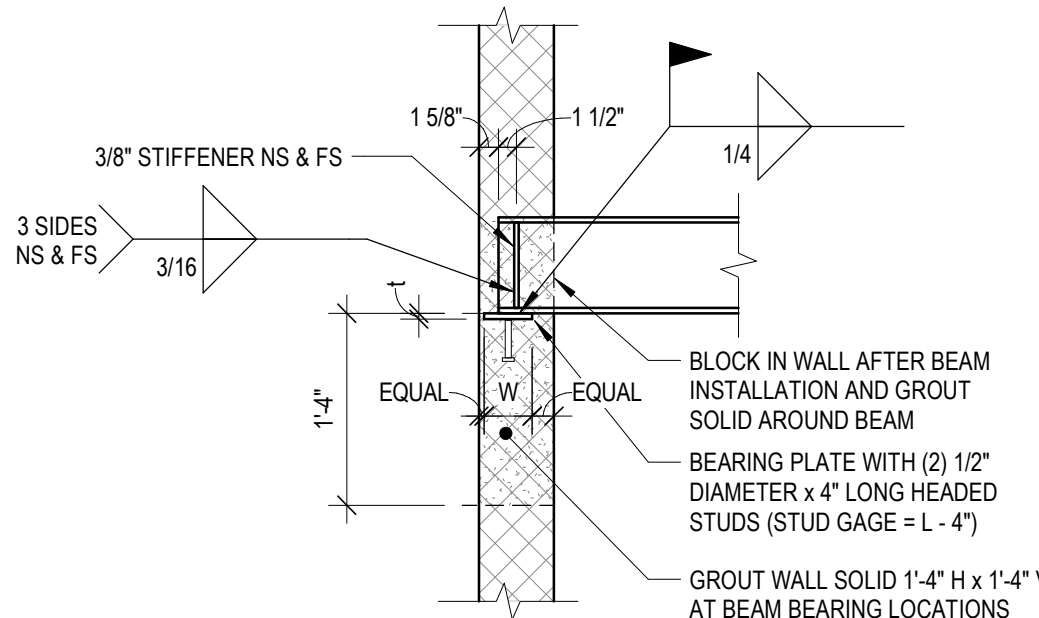


TYPE A



TYPE B

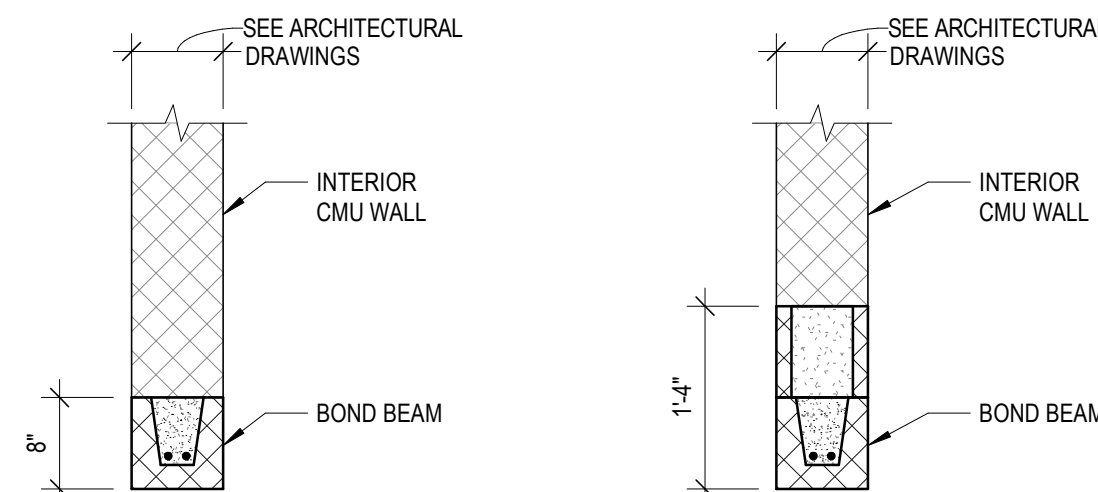
MASONRY BEARING PLATE SCHEDULE			
MARK	PLATE THICKNESS, T (IN)	PLATE WIDTH, W (IN)	PLATE LENGTH, L (IN)
MPB1	1/2"	5'-1/2"	8'



LINTEL SCHEDULE					
MARK	TYPE	SIZE	BEARING LENGTH		REMARKS
N/A	I	8" BOND BEAM WITH (2) #5 BOTTOM	8"		ALL INTERIOR NON-LOAD BEARING WALL OPENINGS LESS THAN OR EQUAL TO 6'-0". SEE DETAIL A.
N/A	II	16" BOND BEAM WITH (2) #5 BOTTOM (FOR OPENINGS LESS THAN OR EQUAL TO 10'-0")	8"		ALL INTERIOR NON-LOAD BEARING WALL OPENINGS GREATER THAN 6'-0" BUT LESS THAN OR EQUAL TO 12'-0". SEE DETAIL A.
L1	I	8" BOND BEAM WITH (2) #5 BOTTOM	8"		SEE DETAIL B
L2	II	16" BOND BEAM WITH (2) #5 BOTTOM	8"		SEE DETAIL B

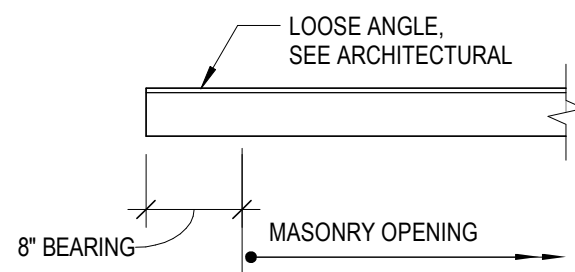
LINTEL SCHEDULE NOTES:

1. SEE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS.
2. COORDINATE BOTTOM OF LINTEL ELEVATION WITH ARCHITECTURAL DRAWINGS.
3. ALL DIMENSIONS ARE NOMINAL MASONRY DIMENSIONS UNLESS NOTED OTHERWISE.
4. PROVIDE MINIMUM 6" BEARING EACH END UNLESS NOTED OTHERWISE.
5. FOR PRECAST CONCRETE LINTELS, WIDTH OF LINTEL = NOMINAL MASONRY WALL THICKNESS - 3/8".
6. FOR CMU LINTELS, CONTRACTOR TO PROVIDE TEMPORARY SHORING UNTIL MASONRY HAS PROPERLY SET (3 DAYS MINIMUM).
7. FOR STEEL LINTELS, PROVIDE 1/4" BOTTOM PLATE UNLESS NOTED OTHERWISE. WIDTH OF PLATE = NOMINAL MASONRY THICKNESS (INCLUDING VENEER) - 1" EXTEND PLATE FULL LENGTH OF LINTEL UNLESS NOTED OTHERWISE.
8. FOR STEEL LINTELS GREATER THAN OR EQUAL TO 12'-0" LONG, PROVIDE 1/2" DIAMETER x 4" LONG HEADED WELDED STUDS AT 32" OC ON TOP FLANGE. STEEL LINTELS LESS THAN 10'-0" LONG MAY BE PLACED LOOSE WITHOUT ANCHOR BOLTS OR BEARING PLATES, UNLESS NOTED OTHERWISE.
9. ALL STEEL LINTELS TO HAVE Fy = 50 KSI.
10. PROVIDE LOOSE LINTELS PER ARCHITECTURAL. SEE TYPE III AND DETAIL A.

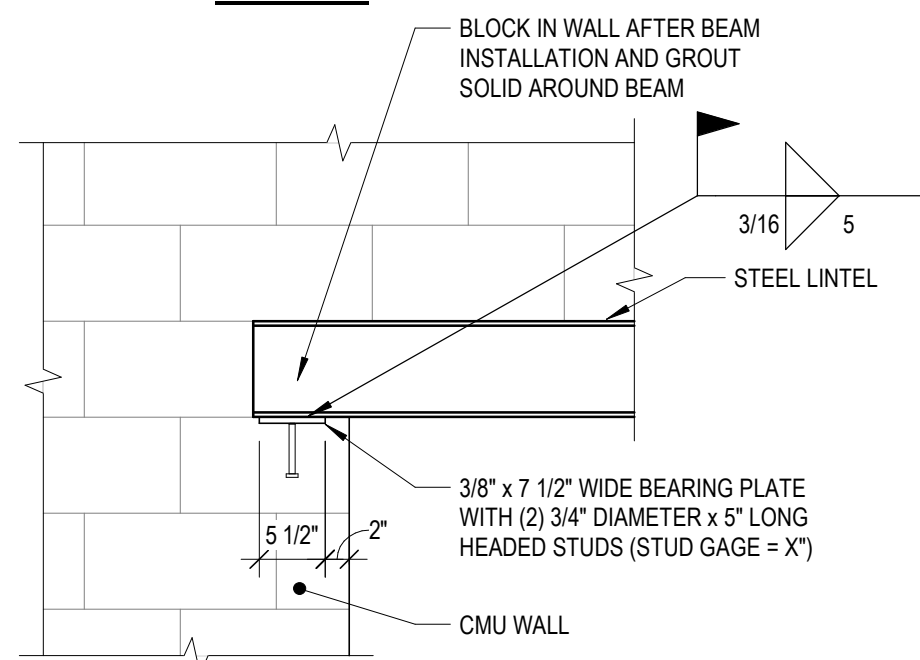


TYPE I

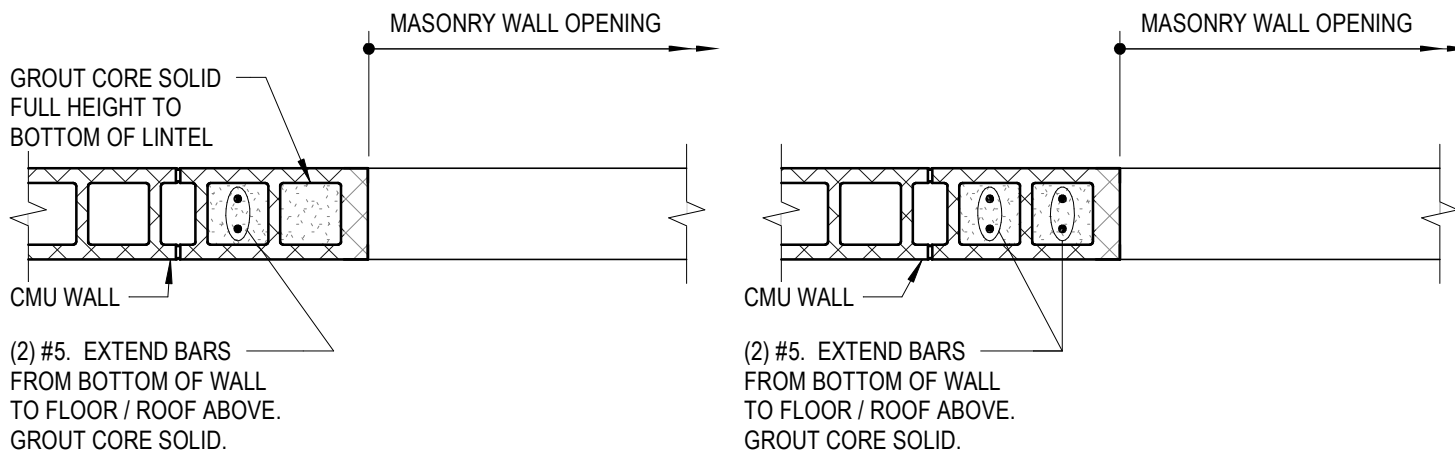
TYPE II



TYPE III



STEEL LINTEL BEARING ELEVATION



DETAIL A

DETAIL B

Gräef

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Suite 202
Madison, WI 53703
608 / 242 1550

www.graef-usa.com

CONSULTANTS:

CLIENT:

Community Development Authority of
the City of Madison



Madison Municipal Building
215 Martin Luther King Jr Blvd
Suite 161
Madison, WI 53703

PROJECT TITLE:

VILLAGE ON PARK PARKING
STRUCTURE AND SITE
IMPROVEMENTS

808 HUGHES PLACE
MADISON, WI 53713

ISSUE:

07/28/2023 BID DOCUMENTS

PROJECT INFORMATION:

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SHEET TITLE:

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SHEET NUMBER:

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